

**DYNFLOW VERSION 5.18:
TESTING AND EVALUATION
OF CODE PERFORMANCE**

by

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1. INTRODUCTION

While at the International Ground Water Modeling Center (IGWMC), Paul van der Heijde (currently an independent consultant working under the name Heath Hydrology out of Boulder, Colorado) has developed a four phase, three-level code testing approach, aimed at characterizing functionality and performance of ground water modeling codes (van der Heijde and Kanzer, 1996). This systematic approach to code testing combines elements of error-detection, evaluation of the operational characteristics of the code, and assessment of its suitability to solve certain types of management problems, with dedicated test problems, relevant test data sets, and informative performance measures. The results of code testing are expressed in terms of correctness (*e.g.*, in comparison with a benchmark), reliability (*e.g.*, convergence and stability of solution algorithms, and absence of terminal failures), efficiency of coded algorithms (in terms of numerical accuracy versus code execution time, and memory and mass storage requirements), and resources required for model setup and analysis (*e.g.*, input preparation time and effort needed to make output ready for graphic analysis) (van der Heijde and Kanzer, 1996). A revised version of this protocol has been adapted in 1996 by the American Society of Testing and Materials (ASTM) as D-6025: *Standard Guide for Developing and Evaluating Ground-Water Modeling Codes*.

According to the protocol described in ASTM D-6025, code testing should sequentially follow the following series of steps:

1. analysis of the code's functionality in terms of simulation functions, operational features, mathematical framework, and software implementation;
2. identification of potential code performance issues based on analysis of simulated processes, mathematical solution methods, computer limitations, and execution environment;
3. development of a code testing strategy and test problems that address relevant code performance issues as they are viewed by all stakeholders (*e.g.*, researchers, code developers, code users, fund managers, regulatory decision makers, project decision makers, etc.);
4. execution of tests and analysis of results using appropriate, comprehensive, informative and accurate graphic and statistical techniques;
5. collection of code information issues and code test problem objectives in overview tables and matrix displays reflecting completeness of testing, as well as correctness, accuracy, efficiency and field applicability of the tested code;
6. identification of performance strengths and weaknesses of the code and the testing procedure;
7. documentation of test objectives, model setup for both the tested code, and the benchmark (structure, discretization, parameters), and results for each test (for both the tested code and the benchmark) in report form and as electronic files including input data, computational

results, statistical analysis of computed results, and graphical representation of key results;
and

8. preparation of an executive summary of functionality, test strategy and results.

The protocol makes a distinction between functionality analysis, and performance evaluation. *Functionality analysis* involves the identification and description of the functions of a simulation code in terms of model framework geometry, simulated processes, boundary conditions, and analytical capabilities, and the subsequent evaluation of each code function or group of functions for conceptual correctness and computational accuracy (including convergence for a practical range of parameter values) and consistency (including numerical stability). The information generated by functionality analysis is organized into a summary structure, or matrix, that brings together the description of code functionality, code-evaluation status, and appropriate test problems. The functionality matrix is formulated combining a complete description of the code functions and features with the objectives of targeted test problems. The functionality matrix illustrates the extent of the performed functionality analysis.

Performance evaluation is aimed at characterizing the operational characteristics of the code in terms of: 1) correctness, 2) overall accuracy; 3) reliability; 4) sensitivity for grid orientation and resolution, and for time discretization; 5) efficiency of coded algorithms (including bandwidth, rate of convergence, memory usage, and disk I/O); and 6) level of effort and resources required for model setup and simulation analysis. Results of the performance evaluation are expressed both quantitatively and qualitatively in checklists and in tabular form. Reporting on performance evaluation should provide potential users information on the performance as a function of problem complexity and setup, selection of simulation control parameters, and spatial and temporal discretization.

The functionality matrix and performance tables, together with the supporting test results and comments, should provide the information needed to select a code for a site-specific application and to evaluate the appropriateness of a code used at a particular site.

The code testing strategy represents a systematic, efficient approach to the comprehensive testing of the code. The code testing strategy includes:

1. formulation of test objectives (as related to code functionality), and of test priorities;
2. selection and/or design of test problems and determination of type and extent of testing for selected code functions or application-dependent combinations of code functions;
3. determination of level of effort to be spent on sensitivity analysis for each test problem;
4. selection of the qualitative and quantitative measures to be used in the evaluation of the code's performance; and

5. determination of the level of detail to be included in the test report and the format of reporting.

The protocol distinguishes four sequentially performed test phases:

1. conceptual testing -- highly simplified problems focused on testing individual functions, features, algorithms or code segments without necessarily comparing its results with a benchmark;
2. benchmarking or verification -- comparing results obtained with a computer code against independently derived solutions such as analytical solutions;
3. code intercomparison -- comparing compatible codes using hypothetical problems or synthetic data sets for which no independent solution is available; and
4. field or laboratory demonstration -- testing against well-documented field or laboratory experiments.

An important aspect of code testing is the definition of informative and efficient measures for use as evaluation or performance criteria. Such measures should characterize quantitatively the differences between the results derived with the simulation code and the benchmark, or between the results obtained with two comparable simulation codes. Evaluation of code testing results should be based on: 1) visual inspection of the graphical representation of variables computed with the numerical model and its benchmark; and 2) quantitative measures of the goodness-of-fit. Graphical measures are especially significant for test results that do not lend themselves to statistical analysis. For example, graphical representation of solution convergence characteristics may indicate numerical oscillations and instabilities in the iteration process. Statistical analysis typically consists of standard linear regression statistics and estimation of error statistics performed on paired data.

This report concerns the testing of the ground-water flow modeling code DYNFLOW (version: 5.18; release date: September 1994), developed by Camp Dresser & McKee, Cambridge, Massachusetts. The related documentation has as publication date April 1994. This report addresses two phases of testing: 1) evaluation of the testing performed by code authors in the past as described in the code's documentation, and 2) additional testing of the code by the author of this test report in cooperation with code's authors. As part of the evaluation of the performed testing, the code has been rerun using the data files prepared by the authors. This report provides an overview of the DYNFLOW functionality, summarizes the completeness of the earlier performed tests, identifies additional testing requirements, and analyzes the results of the performed tests. It includes discussion of the following activities:

1. identifying and examining code functionality;
2. determining type and objectives of tests performed and documented by the code developers;

3. compiling protocol summary structures (*i.e.*, checklists, matrices) using the performed tests;
4. evaluating test results as documented by code developers and reconstructed for this report;
5. identifying possible gaps in the test strategy used by the code developers and designing additional tests to address them; and
6. executing and evaluating the additional tests.

The code-evaluation tests for DYNFLOW were performed on a 200 MHZ Intel Pentium Pro based personal computer using Microsoft Windows NT 4.0. Evaluation plots were prepared with Golden Software's Grapher™ for Windows (version 1.0; Golden Software, 1992) and Microsoft Excel 95 and Excel 97.

2. TESTING DYNFLOW

2.1. CODE DESCRIPTION

The functionality of DYNFLOW has been determined using the generic functionality description form of van der Heijde and Kanzer, 1996, Appendix B; the results are presented in Appendix A of this report. A short description of DYNFLOW is given in the following paragraphs.

DYNFLOW is a general-purpose finite element code designed to simulate transient and steady-state three-dimensional saturated ground water flow under confined and unconfined conditions. Parts of the model domain may change in time between confined and unconfined conditions. The code supports both areal and cross-sectional two-dimensional simulations by reducing the third dimension accordingly. The code may be used for characterizing large, complex, multi-layered, fully-saturated, porous hydrogeologic systems. The code can be used in a quasi-three-dimensional mode.

The flow equation is posed in terms of piezometric head. It is assumed that fluid density does not change during the simulations. DYNFLOW includes calculation of a comprehensive, model-wide mass balance for flow, as well as node-specific fluxes (total net flux, fixed head flux, pumping flux, element recharge flux, and storage flux). Boundary conditions include specified head, specified flux, distributed recharge (at the water table), and evapotranspiration (based on evaporation potential and extinction depth). Head-dependent flux boundaries (*e.g.*, for leakage to or from streams, and flow to drains) are handled by including an extra layer of elements representing the stream/drain resistance, if present. The code includes "rising water" and "dry" boundary option to allow for water-table changes. Springs, seeps, and intermittent streams can be represented by the rising water condition. If a well is simulated in more than one layer, flow can be directly apportioned to the open layers, or represented using one-dimensional elements. Initial conditions can be specified through the input command file, or as a separate file containing heads calculated in previous runs (restart option).

Spatially variable flow parameters include hydraulic conductivity, specific storage or specific yield, recharge (applied only at water table), bulk specific gravity of overburden, and effective stress. The code handles general anisotropy in hydraulic conductivity (parallel to principal directions where principle directions do not have to coincide with Cartesian axes), stress-dependent hydraulic conductivity and specific storativity.

The DYNFLOW code employs linear finite elements, using any combination of four type of elements:

1. three-dimensional elements to represent flow in three directions (triangular discretization in the horizontal with orthogonal discretization in the vertical resulting in elements of varying thickness);
2. two-dimensional elements to represent vertical barriers such as faults and slurry walls;

3. one-dimensional elements to represent multi-aquifer wells, underdrains and fractured rock interconnections; and
4. pond elements to represent surface water bodies.

A typical DYNFLOW grid consists of a number of triangular elements in the horizontal direction, and one or more layers of variable thickness to represent the vertical dimension, resulting in column-shape three-dimensional elements bounded by parallel vertical lines. Elements can be grouped in up to 100 property zones. Element properties include hydraulic conductivity in the principal directions, the rotation angles for principal directions about the Z- and Y-axes, bulk specific gravity of overburden, and effective stress at which the properties are valid.

DYNFLOW includes 4 solvers: 1) Gauss, 2) SOR, 3) hybrid Gauss-SOR, and 4) ICCG. In the transient case, either a trapezoidal (Crank-Nicholson) or implicit time-stepping scheme may be used. The code is operated using user-specified commands (it uses an internal command language structure).

DYNFLOW is dimensionally consistent; any consistent set of units can be used.

The users manual contains sections on model theory and solvers, code operation and command processing, grid design and model formulation, representing stratigraphy, boundary condition processing, simulation of special features, run controls, analysis of output, use of units, calibration, code verification, and example problems.

2.2. TEST ISSUES

Based on the analysis of the functionality of the DYNFLOW code, a list of topics for further evaluation and testing has been compiled (see Table 1). Some of the listed topics reflect specific DYNFLOW functions (represented by program commands and input parameters); others are addressed by DYNFLOW through model formulation and construction (i.e., grid design and parameter allocation). For selected test topics, performance issues are formulated in terms of functional correctness (program logic, terminal failure, inconsistent results), accuracy, and stability; for other test issues objectives are stated in terms of applicability (availability, efficiency, completeness, and clarity). Tables 2A, 2B and 2C present the functionality issues, test objectives, DYNFLOW representation, and testing type for the code's general features, for hydrologic zoning, parameterization and flow characteristics, and boundary conditions, respectively.

The functionality items, listed in the first column of Tables 2A-2C, are represented by explicit code functions, or are addressed during model formulation and construction. The second column of Tables 2A-2C lists a qualifier for the test objective(s) for the selected function. The main (qualitative) objectives are: 1) correctness of computation(s), 2) accuracy of computation(s), 3) completeness of reported information, 4) stability (of numerical solution), and 5) applicability (how easy/difficult is it to use the code to represent particular function). The last column indicates the testing method.

Table 1. Test topics for the three-dimensional finite-element saturated flow code DYNFLOW.

General Features

- code operation (program controls, command structure, I/O instructions)
- output (completeness, clarity, format and layout, suitability for postprocessing)
- grid issues (grid geometry, element types, node/element numbering, automatic bandwidth reduction)
- element superposition (e.g., for multi-layer wells)
- stress superposition (multiple stresses at same location)
- solver(s) operation, accuracy, stability
- time stepping (step size, multiplier, stress period)
- mass balances (regular versus irregular grid; global, zonal, layer-wide, nodal)
- error trapping and clarity of error messages
- file processing (input files, output files, grid files)

Hydrogeologic Zoning, Parameterization, and Flow Characteristics

- steady-state flow
- transient flow
- horizontal flow
- vertical flow
- radial-symmetrical flow
- parameter zones (heterogeneity)
- aquifer pinchout, aquitard pinchout, dipping beds, outcrops, subcrops, and unconformities
- variable thickness layers
- storativity conversion in space and time (confined-unconfined)
- anisotropy (horizontal, vertical)
- changing principal directions
- unconfined conditions (rising/falling water table)
- sharp contrast in hydraulic conductivity (magnitude, principal directions, anisotropy)
- internal no-flow areas
- fracture zones (high conductivity; superposition)
- slurry walls (low conductivity; superposition)
- stress-dependent properties

Boundary Conditions for Flow

- default no-flow assumption (lateral, lower/upper boundaries)
- prescribed head (varying flux)
- prescribed flux (varying head)
- areal recharge in top active cells (water-table)
- induced infiltration from streams/sourcebed aquifer (leaky boundary) with potential for dewatering below the base of the semi-pervious boundary (constant flux/depth dependent flux)
- induced exfiltration towards stream
- drain boundary (no b.c. if g.w. head is below drain level)
- reservoir boundary (extent varies in time)
- model dries up (head below bottom of model domain)
- irregular geometry and internal no-flow regions
- time-varying discharging/ recharging wells
- multi-aquifer/multi-model-layer wells
- (depth-limited) evapotranspiration
- pond element
- springs/seeps
- seepage face

Table 2-A. Functionality Issues for General Features.

<i>Functionality Issue</i>	<i>Test Objective(s)</i>	<i>DYNFLOW Implementation/representation</i>	<i>DYNFLOW Testing Method</i>
Program controls/command structure	correctness	code function	runtime testing
I/O instructions	correctness	code function	runtime testing
Form of output	completeness clarity format	code function	manual evaluation
Grid/mesh issues: element/node numbering (bandwidth reduction)	correctness	code function	runtime testing
Grid/mesh issues: element superposition (of 1D and 2D elements on 3D mesh)	correctness	code function	runtime testing
Stress superposition at individual nodes and time steps (pumping, recharge, evapotranspiration)	correctness	code function	verification
Solver performance (including sensitivity to iteration criteria, acceleration parameters, etc.)	correctness, accuracy, stability	code function	runtime testing, verification
Time stepping (specification of time step, multiplication factor, stress period)	correctness	code function	runtime testing
Mass balance (global, zonal, layer-wide, nodal)	applicability, completeness	code function	evaluation
Error trapping	correctness completeness	code function	manual evaluation, crash testing
File processing	correctness	code function	runtime testing

Table 2-B. Functionality Issues for Hydrogeologic Zoning, Parameterization, and Flow Characteristics.

<i>Functionality Issue</i>	<i>Test Objective(s)</i>	<i>DYNFLOW Implementation/representation</i>	<i>Testing Method</i>
Steady-state vertical flow	correctness, accuracy	code function	verification
Steady-state horizontal flow	correctness, accuracy	code function	verification
Steady-state three-dimensional flow	correctness, accuracy	code function	verification
Transient vertical flow	correctness, accuracy	code function	verification
Transient horizontal flow	correctness, accuracy	code function	verification
Transient radial flow	correctness, accuracy	code function	verification
Transient three-dimensional flow	correctness, accuracy	code function	verification
Multiple parameter zones to represent heterogeneity	correctness	code function	verification, inter-comparison
Representing aquifer pinchout, aquitard pinchout, dipping beds, outcrops, subcrops, and unconformities	applicability	model construction	inter-comparison
Representing variable thickness layers	correctness, stability	code function	verification, inter-comparison
Storativity conversion in space and time. When the head in a section of a confined aquifer drops below the top of that aquifer in that area, conditions reverse to unconfined, and vice versa. Accordingly, the code needs to switch between confined storativity and unconfined storativity (specific yield).	correctness, accuracy, stability	code function	verification, inter-comparison
Horizontal anisotropy	correctness, accuracy	code function	verification

<i>Functionality Issue</i>	<i>Test Objective(s)</i>	<i>DYNFLOW Implementation/ representation</i>	<i>Testing Method</i>
Vertical anisotropy	correctness, accuracy	code function	verification, inter-comparison
Element-wise changing principal directions	correctness, accuracy	code function	inter-comparison
Unconfined flow. In an unconfined aquifer, transmissivity/conductance is a function of saturated thickness, and thus dependent on the computed heads.	correctness, accuracy, stability	code function	verification, inter-comparison, demonstration
Unconfined flow. In multi-layer models of unconfined aquifers, a rising water table might arise above the initial saturated/wetted model layers, invading dry cells (resaturation/wetting).	correctness, accuracy, stability	code function	inter-comparison, demonstration
Unconfined flow. In multi-layer models of unconfined aquifers, a falling water table might drop below the bottom of the initial (partially) water-filled cells (desaturation).	correctness, accuracy, stability	code function	inter-comparison, demonstration
Unconfined flow. A declining head may (non-) deliberately drop below the bottom of model domain.	correctness, stability	code function	consistency inspection
Internal no-flow areas and their (lack of) influence on results (heads, mass balance)	applicability, correctness	model formulation (grid design or parameter allocation)	inter-comparison
Sharp contrast in hydraulic conductivity of neighboring elements (lateral)	accuracy, stability	code function	inter-comparison, consistency inspection
Sharp contrast in hydraulic conductivity of neighboring layers	accuracy, stability	code function	inter-comparison, consistency inspection
Simulation of thin, conductive zones such as fracture zones and underdrains (see also superposition and 1D elements in Table 2A)	applicability	model construction	inter-comparison

<i>Functionality Issue</i>	<i>Test Objective(s)</i>	<i>DYNFLOW Implementation/ representation</i>	<i>Testing Method*</i>
Simulation of thin barriers such as slurry walls and faults (see also superposition and 2D elements in Table 2A)	applicability	model construction	inter-comparison
Stress dependent properties (hydraulic conductivity, effective porosity, storativity, specific yield)	correctness, accuracy	code function	verification, inter-comparison

*) consistency inspection refers to visual inspection of results for physical and mathematical consistency.

Table 2-C. Functionality Issues for Boundary Conditions.

<i>Functionality Issue</i>	<i>Test Objective(s)</i>	<i>DYNFLOW Implementation/ representation</i>	<i>DYNFLOW Testing*</i>
Default no-flow (lateral, upper/lower boundary)	correctness, accuracy	code function	verification
Prescribed head (steady-state, transient)	correctness, accuracy	code function	verification
Prescribed flux (steady-state, transient)	correctness, accuracy	code function	verification
Areal recharge in top active elements (steady-state, transient)	correctness, accuracy	code function	verification, inter-comparison
Induced infiltration from stream or sourcebed aquifer (head-dependent varying flux)	applicability	model construction	verification, inter-comparison
Induced infiltration from stream or sourcebed aquifer (head-independent constant flux)	applicability	model construction	verification, inter-comparison
Switching between induced infiltration (from stream) and exfiltration (towards stream)	applicability	model construction	inter-comparison, consistency inspection

<i>Functionality Issue</i>	<i>Test Objective(s)</i>	<i>DYNFLOW Implementation/ representation</i>	<i>DYNFLOW Testing*</i>
Drain boundary (no boundary condition if head is below drain level)	correctness	code function	inter-comparison, consistency inspection
Reservoir boundary (extent of surface water body in contact with ground-water varies over time; head-dependent flux)	applicability	model construction	inter-comparison
Model dries up (head locally declines below model bottom)	correctness error message	code function	inter-comparison, consistency inspection
No flow regions within model area (represented by inactive elements, zero h.c. elements, or excluding it from model domain by creating internal boundary)	applicability	model construction code function	inter-comparison, consistency inspection
Recharging/discharging wells	correctness, accuracy	code function	verification
Multi-aquifer wells (filter in multiple model layers)	correctness, accuracy	code function	verification, inter-comparison
Depth-limited evapotranspiration	correctness, accuracy	code function	verification, inter-comparison
Pond element	correctness, accuracy	code function	verification, inter-comparison
Springs/seeps	applicability	model construction	verification, inter-comparison
Seepage face	applicability	model construction	verification, demonstration

*) consistency inspection refers to visual inspection of results for physical and mathematical consistency.

2.3. TESTS DISCUSSED IN DOCUMENTATION

The identified test issues should be evaluated through a well-chosen set of benchmark and inter- and intracomparison tests. The documentation of Version 5.18 of DYNFLOW includes ten verification tests using independent benchmarks (see Table 3 and Table 4). Some of the tests have been set up to facilitate code intracomparison (e.g., DFTC03). For a number of tests, multiple runs were made reflecting alternative implementation of the benchmark with DYNFLOW. To assess the accuracy of the documented tests, the tests have been run again as part of this evaluation; the results are summarized in Appendix B, test cases DFTC01-DFTC09. Some DYNFLOW reference materials mention additional test cases (Wetting Front, Spherical, and Recharge & ET). These test cases have not been evaluated as part of the first phase of this code evaluation project because they were not described in the documentation available during the project. However, they have been included in the second phase (see section 2.4). It should be noted that the testing of DYNFLOW focused on test issues related to functions explicitly represented in DYNFLOW by commands and input parameters. Issues related to model formulation and construction have not been evaluated as part of this code evaluation project. All tests were performed using the ICCG solver, unless mentioned otherwise. The ICCG solver appears to be the most universal solver available in DYNFLOW. Performance of other available DYNFLOW solvers has not been included in this report.

Table 3. Tests discussed in DYNFLOW Documentation of April 1994.

DYNFLOW Documentation		This Report	
Test No.	Name	Test No.	Name
1.	Linear	DFTC01	Linear 1-D Flow
2.	Dupuit	DFTC02	1-D Dupuit (Non-Linear) Flow
3.	Tank	DFTC03	Tank/Pond Drainage
4.	Theis	DFTC04	Theis Curves
5.	Hantush	DFTC05	Hantush Solution
6.	Consolidation	DFTC06	Consolidation
7.	Pond	see test DFTC03	
8.	Prickett	DFTC09	Confined-Unconfined Storage Conversion
9.	Seepage	DFTC07	Seepage Face
10.	Mound	DFTC08	Mound

Table 4: List of code tests and example applications presented in the DYNFLOW documentation, version April 1994.

Reference Number	Name	Description	Test Objective(s)	Type of Benchmark
DFTC01A	Linear	Steady-state, one-dimensional horizontal flow between two fully penetrating parallel drains in homogeneous confined aquifer	<ul style="list-style-type: none"> horizontal confined flow under steady-state conditions in isotropic aquifer fixed head b.c. 	analytical solution
DFTC01B			<ul style="list-style-type: none"> horizontal anisotropy horizontal rotation of principle axes 	intra-comparison (partial testing)
DFTC01C			<ul style="list-style-type: none"> rotation of principle axes in vertical plane 	intra-comparison (partial testing)
DFTC02	Dupuit	Steady-state, one-dimensional unconfined horizontal flow to a fully-penetrating parallel drains	<ul style="list-style-type: none"> horizontal unconfined flow under steady-state conditions in isotropic aquifer unconfined condition in a multi-layer system fixed head b.c. 	analytical solution
DFTC03A	Tank Drainage	Transient, one-dimensional uniform vertical flow (gravity drainage) under unconfined conditions	<ul style="list-style-type: none"> vertical flow and position phreatic surface under transient conditions 	analytical solution; intra-comparison
DFTC03B			<ul style="list-style-type: none"> performance of 1D element 	
DFTC03C			<ul style="list-style-type: none"> performance of POND element 	
DFTC04	Theis	Transient, radial horizontal flow to a fully-penetrating well in an infinite, isotropic, confined aquifer of constant thickness	<ul style="list-style-type: none"> transient horizontal flow under confined conditions (head and flux) discharging well lateral fixed head b.c. 	analytical solution
DFTC05	Hantush	Transient, radial horizontal flow to a fully-penetrating well in an infinite, isotropic, leaky-confined aquifer nodal flux.	<ul style="list-style-type: none"> transient horizontal and vertical flow under confined conditions (DF is fully 3D) lateral fixed head b.c. upper boundary fixed head b.c. discharging well 	analytical solution
DFTC06	Consolidation	Transient, confined, one-dimensional vertical flow (free drainage) in aquifer subject to consolidation	<ul style="list-style-type: none"> transient vertical flow under confined conditions (free drainage) fixed head at lower boundary stress-varying permeability and porosity 	analytical solution

Reference Number	Name	Description	Test Objective(s)	Type of Benchmark
DFTC07	Seepage	Steady-state, phreatic, one-dimensional horizontal flow with a seepage face	<ul style="list-style-type: none"> • steady-state unconfined horizontal flow • rising water algorithm • fixed head lateral boundary conditions 	analytical solution
DFTC08	Mound	Steady-state, phreatic, one-dimensional horizontal flow between two drains with uniform recharge.	<ul style="list-style-type: none"> • seepage face • steady-state horizontal unconfined flow • lateral fixed head boundary condition • areal recharge 	analytical solution
DFTC09	Confined-Unconfined Conversion	Transient, horizontal radial flow towards pumping well in homogeneous, isotropic aquifer under mixed confined/unconfined conditions	<ul style="list-style-type: none"> • transient horizontal (radial) flow • well discharge function • unconfined/confined storage factor conversion • unconfined/confined piezometric surface 	analytical solution

The documented tests are well-designed from a verification perspective. Although incompletely documented, they are addressing major (but not all) code functions. Reviewing for completeness of testing, it appears that a number of test issues were not addressed by the code authors. These outstanding issues include:

- fully threedimensional flow;
- effects of high contrast in parameter values for neighboring elements (both lateral and vertical);
- desaturation below model or aquifer bottom;
- recharge and evapotranspiration;
- seepage face and resaturation (wetting front);
- multi-layer wells;
- combinations of stresses (recharge plus wells plus ET); and
- transient boundary conditions and stresses.

2.4. ADDITIONAL TESTS

To address some of the test issues not discussed in the code documentation, five additional tests were run (see Table 5). All additional tests were performed using the ICCG solver, as was the case with the first set of tests. The results are summarized in Appendix B, test cases DFTC10-DFTC14.

Table 5. List of additional DYNFLOW code tests.

Reference Number	Name	Description	Test Objective(s)	Type of Benchmark
DFTC10	Rising/Falling Water Table	Transient flow in a multi-layer, rectangular isotropic, homogeneous, unconfined aquifer; flow starts after sudden rise of fixed head boundary condition at one side of the semi-infinite model domain.	<ul style="list-style-type: none"> unconfined potentiometric surface calculation (correctness/accuracy) (re-)wetting of dry elements by rising water table (correctness/stability) 	analytical solution
DFTC11	Spherical Flow	Steady-state flow away from an injection well with an infinite small screen located in the center of a three-dimensional infinite domain; the recharge rate is held constant; the aquifer is isotropic, homogeneous and confined.	<ul style="list-style-type: none"> fully three-dimensional steady-state flow under confined conditions (i.e., symmetrical flow) fixed head condition at the lateral, top and bottom boundaries recharging well with constant flux mass balance calculations 	analytical solution
DFTC12	Recharge and ET	Steady state horizontal uniform flow between two fully penetrating drains in an isotropic, homogeneous, unconfined aquifer with a horizontal impermeable base modified by recharge and evaporation.	<ul style="list-style-type: none"> distributed recharge and evaporation algorithms and their additive use. 	analytical solution for recharge; manual evaluation of global water balance and local evaporation.

Reference Number	Name	Description	Test Objective(s)	Type of Benchmark
DFTC13	Heterogeneity	Steady state confined flow towards a fully penetrating well in a dual transmissivity, isotropic aquifer of infinite extent with a horizontal impermeable base. The two transmissivity zones are separated by a straight vertical plane (the Y-Z plane).	<ul style="list-style-type: none"> • assignment of properties • correct representation of heterogeneity. 	analytical solution
DFTC14	Anisotropy	Transient flow towards a fully penetrating pumping well with a constant discharge rate in an anisotropic, homogeneous, confined porous medium of infinite extent and constant thickness.	<ul style="list-style-type: none"> • transient horizontal flow under confined anisotropic conditions • effects of discharging well • influence of lateral fixed head boundary condition 	analytical solution

3.0 CONCLUSIONS

This report concerns the testing and evaluation of the ground-water flow modeling code DYNFLOW (version: 5.18; release date: September 1994), developed by Camp Dresser & McKee, Cambridge, Massachusetts. The testing and evaluation of DYNFLOW was carried out in two phases: 1) execution and evaluation of the tests described in the code's documentation, and 2) additional testing of the code conform procedures outlined in ASTM Standard D-6025. After analyzing DYNFLOW's functionality, the completeness of the earlier performed tests was assessed, upon which additional testing requirements were formulated and executed. The results of the tests have been documented in Appendix B of this report. The issues listed in table 2A for runtime testing have been addressed during the execution of the test cases. There were no performance problems regarding these issues.

The set of fourteen tests are adequate to test DYNFLOW's internal functionality, that is, those functions that are represented by coded routines. The results of the tests show that, if model construction and parameterization is carefully handled, the DYNFLOW code performs satisfactory. The quality of the results is very dependent on spatial and temporal discretization, as is typical for numerical simulation codes. This means that in its application, DYNFLOW requires careful design of grid structure and selection of time step sequence, a notion recognized by the code authors as reflected by the attention given to these issues in the documentation.

4. REFERENCES

van der Heijde, P. K. M., and Kanzer, D. A. (1995) Ground-Water Model Testing: Systematic Evaluation and Testing of Code Functionality and Performance. Submitted to U.S. Environmental Protection Agency under CR-818719, International Ground Water Modeling Center, Golden, Colorado.

Appendix A: DYNFLOW FUNCTIONALITY TABLE

GROUND WATER MODEL FUNCTIONALITY DESCRIPTION

MODEL NAME: DYNFLOW
 VERSION: 5.18
 RELEASE DATE: SEPTEMBER 1996

AUTHOR(S):
 INSTITUTION OF DEVELOPMENT: CAMP DRESSER & MCKEE

CONTACT ADDRESS:
 PHONE:
 FAX:

PROGRAM LANGUAGE: FORTRAN 77
 COMPUTER PLATFORM(S): VARIOUS

LEGAL STATUS: PROPRIETARY
 PREPROCESSING OPTIONS: DYNPLOT

POSTPROCESSING FACILITIES: DYNPLOT

MODEL TYPE

- | | | |
|--|--|--|
| <input checked="" type="checkbox"/> single phase saturated flow | <input type="checkbox"/> parameter ID unsaturated flow (analytical/ numerical) | <input type="checkbox"/> sediment transport |
| <input type="checkbox"/> single phase unsaturated flow | <input type="checkbox"/> parameter ID solute transport (numerical) | <input type="checkbox"/> surface water runoff |
| <input type="checkbox"/> vapor flow/transport | <input type="checkbox"/> aquifer test analysis | <input type="checkbox"/> stochastic simulation |
| <input type="checkbox"/> solute transport | <input type="checkbox"/> tracer test analysis | <input type="checkbox"/> geostatistics |
| <input type="checkbox"/> virus transport | <input type="checkbox"/> flow of water and steam | <input type="checkbox"/> multimedia exposure |
| <input type="checkbox"/> heat transport | <input type="checkbox"/> fresh/salt water interface | <input type="checkbox"/> pre-/postprocessing |
| <input type="checkbox"/> matrix deformation | <input type="checkbox"/> twophase flow | <input type="checkbox"/> expert system |
| <input type="checkbox"/> geochemical | <input type="checkbox"/> threephase flow | <input type="checkbox"/> data base |
| <input type="checkbox"/> optimization | <input type="checkbox"/> phase transfers | <input type="checkbox"/> ranking/screening |
| <input type="checkbox"/> groundwater and surface water hydraulics | <input type="checkbox"/> chemical transformations | <input type="checkbox"/> water budget |
| <input type="checkbox"/> parameter ID saturated flow (inverse numerical) | <input type="checkbox"/> biochemical transformations | <input type="checkbox"/> heat budget |
| | <input type="checkbox"/> watershed runoff | <input type="checkbox"/> chemical species mass balance |

UNITS

- | | | |
|--|---|---------------------------------------|
| <input type="checkbox"/> SI system | <input type="checkbox"/> US customary units | <input type="checkbox"/> user-defined |
| <input checked="" type="checkbox"/> metric units | <input checked="" type="checkbox"/> any consistent system | |

PRIMARY USE

- | | | |
|------------------------------------|---|---|
| <input type="checkbox"/> research | <input checked="" type="checkbox"/> general use | <input type="checkbox"/> policy-setting |
| <input type="checkbox"/> education | <input type="checkbox"/> site-dedicated | |

 GENERAL MODEL CHARACTERISTICS

Parameter discretization

- lumped
 - mass balance approach
 - transfer function(s)
- distributed
- deterministic
- stochastic

Spatial orientation

saturated flow

- 1D horizontal
- 1D vertical
- 2D horizontal (areal)
- 2D vertical (crosssectional or profile)
- 2D axi-symmetric (horizontal flow only)
- fully 3D
- quasi-3D (layered; Dupuit approx.)
- 3D cylindrical or radial (flow defined in horizontal and vertical directions)

unsaturated flow

- 1D horizontal
- 1D vertical
- 2D horizontal
- 2D vertical
- 2D axi-symmetric
- fully 3D
- 3D cylindrical or radial

Restart capability - types of updates possible

- dependent variables (e.g., head, concentration, temperature)
- fluxes
- velocities
- parameter values
- stress rates (pumping, recharge)
- boundary conditions

Discretization in space

- no discretization
- uniform grid spacing/regular mesh
- variable grid spacing/irregular mesh
- movable grid (relocation of nodes during run)
- maximum number of nodes/cells/elements
 - modifiable in source code (requires compilation)
 - modifiable through input
- maximum number of nodes (standard version):
- maximum number of cells/elements (standard version):

Possible cell shapes

- 1D linear
- 1D curvilinear
- 2D triangular
- 2D curved triangular
- 2D square
- 2D rectangular
- 2D quadrilateral
- 2D curved quadrilateral
- 2D polygon
- 2D cylindrical
- 3D cubic
- 3D rectangular block
- 3D hexahedral (6 sides)
- 3D tetrahedral (4 sides)
- 3D spherical

FLOW SYSTEM CHARACTERIZATION

Saturated zoneHydrogeologic zoning

- confined
- semi-confined (leaky-confined)
- unconfined (phreatic)
- hydrodynamic approach
- hydraulic approach (Dupuit-Forcheimer assumption for horizontal flow)
- single aquifer
- single aquifer/aquitard system
- multiple aquifer/aquitard systems
 - max. number of aquifers:
- discontinuous aquifers (aquifer pinchout)
- discontinuous aquitards (aquitard pinchout)
- storativity conversion in space (confined-unconfined)
- storativity conversion in time
- aquitard storativity

Hydrogeologic medium

- porous medium
- fractured impermeable rock (fracture system, fracture network)
- discrete individual fractures
- equivalent fracture network approach
- discrete fracture zones
- dual porosity system (flow in fractures and optional in porous matrix, storage in porous matrix and exchange between fractures and porous matrix)
- uniform hydraulic properties (hydraulic conductivity, storativity)
- anisotropic hydraulic conductivity
- nonuniform hydraulic properties (heterogeneous)
- hydraulic properties stress-dependent

Flow characteristics

- single fluid, water
- single fluid, vapor
- single fluid, NAPL
- air and water flow
- water and steam flow
- moving fresh water and stagnant salt water
- moving fresh water and salt water
- water and NAPL
- water, vapor and NAPL
- incompressible fluid
- compressible fluid
- variable density
- variable viscosity
- linear laminar flow (Darcian flow)
- non-Darcian flow
- steady-state flow
- transient (non-steady state) flow
- dewatering (desaturation of cells)
- dewatering (variable transmissivity)
- rewatering (resaturation of dry cells)
- delayed yield from storage

Boundary conditions

- infinite domain
- semi-infinite domain
- regular bounded domain
- irregular bounded domain
- fixed head
- prescribed time-varying head
- zero flow (impermeable barrier)
- fixed cross-boundary flux
- prescribed time-varying cross-boundary flux
- areal recharge:
 - constant in space
 - variable in space
 - constant in time
 - variable in time

Boundary conditions - continued

- induced recharge from or discharge to a source bed aquifer or a stream in direct contact with ground water
 - surface water stage constant in time
 - surface water stage variable in time
 - stream penetrating more than one aquifer
- induced recharge from a stream not in direct contact with groundwater
- evapotranspiration dependent on distance surface to water table
- drains (gaining only)
- free surface
- seepage face
- springs, seeps

Sources/Sinks

- point sources/sinks (recharging/pumping wells)
 - constant flow rate
 - variable flow rate
 - head-specified
 - partially penetrating
 - well loss
 - block-to-radius correction
 - well-bore storage
 - multi-layer well
- line source/sinks (internal drains)
 - constant flow rate
 - variable flow rate
 - head-specified
- collector well (horizontal, radially extending screens)
- mine shafts (vertical)
 - water-filled
 - partially filled
- mine drifts, tunnel (horizontal)
 - water-filled
 - partially filled

Flow System Characteristics -- continued

Dependent variable(s)

- | | |
|--|---|
| <input checked="" type="checkbox"/> head | <input type="checkbox"/> potential |
| <input type="checkbox"/> drawdown | <input type="checkbox"/> moisture content |
| <input type="checkbox"/> pressure | <input type="checkbox"/> stream function |
| <input type="checkbox"/> suction | <input type="checkbox"/> velocity |

Solution methods - Flow

- | | |
|---|--|
| <input type="checkbox"/> analytical | Time-stepping scheme |
| <input type="checkbox"/> single solution | <input checked="" type="checkbox"/> fully implicit |
| <input type="checkbox"/> superposition | <input type="checkbox"/> fully explicit |
| <input type="checkbox"/> method of images | <input checked="" type="checkbox"/> Crank-Nicholson |
| <input type="checkbox"/> analytic element method | Matrix-solving technique |
| <input type="checkbox"/> point sources/sinks | <input checked="" type="checkbox"/> Iterative |
| <input type="checkbox"/> line sinks | <input checked="" type="checkbox"/> SIP |
| <input type="checkbox"/> ponds | <input type="checkbox"/> Gauss-Seidel (PSOR) |
| <input type="checkbox"/> uniform flow | <input type="checkbox"/> LSOR |
| <input type="checkbox"/> rainfall | <input checked="" type="checkbox"/> SSOR |
| <input type="checkbox"/> layering | <input type="checkbox"/> BSOR |
| <input type="checkbox"/> inhomogeneities | <input type="checkbox"/> ADIP |
| <input type="checkbox"/> doublets | <input type="checkbox"/> Iterative ADIP (IADI) |
| <input type="checkbox"/> leakage through confining beds | <input type="checkbox"/> Predictor-corrector |
| <input type="checkbox"/> Semi-analytical | <input checked="" type="checkbox"/> Direct |
| <input type="checkbox"/> continuous in time, discrete in space | <input checked="" type="checkbox"/> Gauss elimination |
| <input type="checkbox"/> continuous in space, discrete in time | <input type="checkbox"/> Cholesky decomposition |
| <input type="checkbox"/> approximate analytical solution | <input type="checkbox"/> Frontal method |
| <input type="checkbox"/> Solving stochastic PDE's | <input checked="" type="checkbox"/> Doolittle |
| <input type="checkbox"/> Monte Carlo simulations | <input type="checkbox"/> Thomas algorithm |
| <input type="checkbox"/> spectral methods | <input type="checkbox"/> Point Jacobi |
| <input type="checkbox"/> small perturbation expansion | <input type="checkbox"/> Iterative methods for nonlinear equations |
| <input type="checkbox"/> self-consistent or renormalization technique | <input type="checkbox"/> Picard method |
| <input checked="" type="checkbox"/> Numerical | <input type="checkbox"/> Newton-Raphson method |
| Spatial approximation | <input type="checkbox"/> Chord slope method |
| <input type="checkbox"/> finite difference method | <input type="checkbox"/> Semi-iterative |
| <input type="checkbox"/> block-centered | <input checked="" type="checkbox"/> conjugate-gradient |
| <input type="checkbox"/> node-centered | |
| <input type="checkbox"/> integrated finite difference method | |
| <input type="checkbox"/> boundary elements method | |
| <input type="checkbox"/> particle tracking | |
| <input type="checkbox"/> pathline integration | |
| <input checked="" type="checkbox"/> finite element method | |

Flow System Characteristics -- continued

Output Characteristics - Flow

Echo of input (in ASCII text format)

- grid (nodal coordinates, cell size, element connectivity)
- initial heads/pressures/potentials
- initial moisture content/saturation
- soil parameters/function coefficients
- aquifer parameters
- boundary conditions
- stresses (recharge, pumping)

Simulation results - form of output

- dependent variables in binary format
- complete results in ASCII text format
- spatial distribution of dependent variable for postprocessing
- time series of dependent variable for postprocessing
- direct screen display - text
- direct screen display - graphics
- direct hardcopy (printer)
- direct plot (pen-plotter)
- graphic vector file
- graphic bit map/pixel/raster file

Simulation results - type of output

- head/pressure/potential
 - areal values (table, contours)
 - temporal series (table, x-t graphs)
- saturation/moisture content
 - areal values (table, contours)
 - temporal series (table, x-t graphs)
- head differential/drawdown
 - areal values (table, contours)
 - temporal series (table, x-t graphs)
- moisture content/saturation
 - areal values (table, contours)
 - temporal series (table, x-t graphs)

Type of output - continued

- internal (cross-cell) fluxes
 - areal values (table, vector plots)
 - temporal series (table, x-t graphs)
- infiltration/recharge fluxes
 - areal values (table, vector plots)
 - temporal series (table, x-t graphs)
- evapo(transpi)ration fluxes
 - areal values (table, vector plots)
 - temporal series (table, x-t graphs)
- cross boundary fluxes
 - areal values (table, vector plots)
 - temporal series (table, x-t graphs)
- velocities
 - areal values (table, vector plots)
 - temporal series (table, x-t graphs)
- stream function values
- streamlines/pathlines (graphics)
- traveltimes (tables)
- isochrones (graphics)
- position of interface (table, graphics)
- location of seepage faces
- water budget components
 - cell-by-cell
 - global (total model area)
- calculated parameters

Computational information

- iteration progress
- iteration error
- mass balance error
- cpu time use
- memory allocation

Appendix B: GROUND-WATER FLOW TEST PROBLEMS

Test 1: LINEAR 1-D FLOW

Test 2: 1-D DUPUIT (NON-LINEAR) FLOW

Test 3: TANK/POND DRAINAGE

Test 4: THEIS CURVES

Test 5: HANTUSH SOLUTION

Test 6: CONSOLIDATION

Test 7: SEEPAGE FACE

Test 8: MOUND

Test 9: CONFINED-UNCONFINED STORAGE CONVERSION

Test 10: RISING/FALLING WATER TABLE (Wetting Front)

Test 11: SPHERICAL FLOW (Partial Penetrating Well in Confined Aquifer)

Test 12: RECHARGE AND ET

Test 13: HETEROGENEITY

Test 14: ANISOTROPY

Test 1: LINEAR

tests #: DFTC01A, DFTC01B, DFTC01C

command files: DFTC01A.CFI, DFTC01B.CFI, DFTC01C.CFI

grid files: DFTC01A.GRF, DFTC01B.GRF, DFTC01C.GRF

output files: DFTC01A.OUT, DFTC01B.OUT, DFTC01C.OUT

manual reference: Section G-2

date of test execution: October 28, 1996

title: LINEAR: Horizontal one-dimensional confined (linear) flow

description: Horizontal uniform flow between two fully penetrating drains in an isotropic, homogeneous, confined aquifer of uniform thickness and with horizontal impermeable base (see figure B1-1).

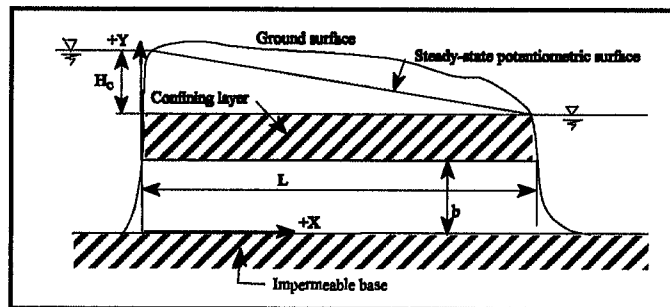


Figure B1-1. Conceptual model for test case DFTC01.

tested functions: DFTC01A: Confined potentiometric surface calculation and node point flux computations for case where principle direction of hydraulic conductivity is parallel with the flow direction; implementation of constant head boundary conditions.
DFTC01B: Horizontal anisotropy algorithm and rotation of principal direction of hydraulic conductivity in the horizontal plane.
DFTC01C: Rotation of principal direction of hydraulic conductivity in the vertical plane.

model domain: Strip between two parallel drains with a length, width and thickness of $L=100$ ft, $w=10$ ft, and $b=20$ ft, respectively.

boundary conditions: Constant head at left boundary ($x=0$ ft) is $h_0=100$ ft; constant head at right boundary ($x=100$ ft) is $h_l=50$ ft; no-flow boundaries in y - and z -direction.

benchmark: Analytical solution for linear potentiometric surface between boundaries and flux calculations based on Darcy's law for confined, one-dimensional flow under steady-state

conditions; discharge is calculated from:

$$Q = -KIA = -K \frac{H_c}{L} bw$$

where, Q = flow rate [L^3/T], K = hydraulic conductivity [L/T], i = hydraulic gradient [L/L], A = cross sectional area of flow [L^2], H_c = head at downstream boundary minus head at upstream boundary [L], L = length of aquifer [L], b = thickness of aquifer [L], and W = width of aquifer [L].

Hydraulic gradient over the length of the aquifer is constant at $i = 0.5$; flow rate in aquifer is constant at $Q = 1,000 \text{ ft}^3/\text{d}$ (i.e., inflow at upstream boundary, outflow at downstream boundary); heads are obtained by linear interpolation between given heads at the lateral (drain) boundaries.

grid: Two-dimensional horizontal grid with one layer and two levels; in plan view, the grid consists of 33 nodes in three rows, defining 40 elements (see figure B1-2). Heads are fixed above the layer to simulate confined conditions; note that the grid for test DFTC01C consist of 36 nodes and 44 elements.

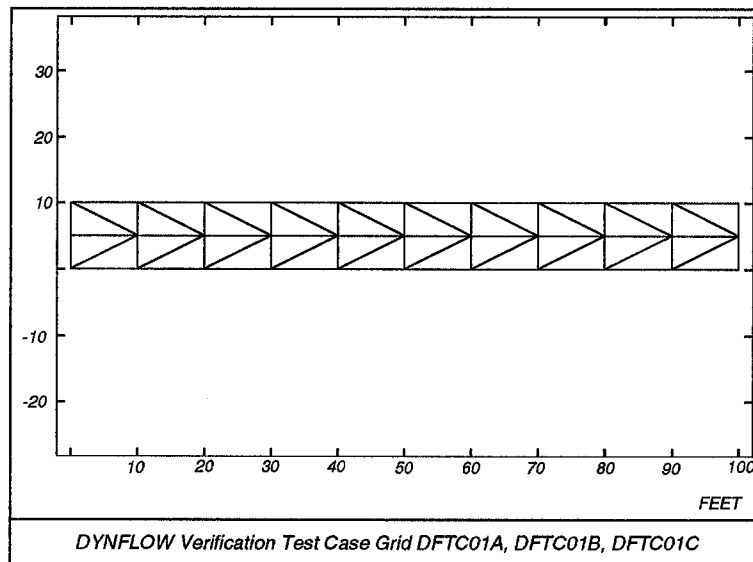


Figure B1-2. DYNFLOW grid for test case DFTC01.

initial conditions: n.a. (steady state)

time-stepping: n.a. (steady state)

system parameters: $K_x = K_y = K_z = 10.0 \text{ ft/d}$

control parameters: Tolerance = .001 ft; alpha = 1.5 (relaxation factor); max. # outer iterations = 10; max. # inner iterations = 90

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Graphic plot of heads in selected locations (see Fig. B1-3); tabular listing of heads (see Table B1-1); no statistical measures calculated in absence of non-zero deviations.

Table B1-1

distance from downstream end (ft)	piezometric head (ft)			
	analytical solution	DYNFLOW DFTC01A	DYNFLOW DFTC01B	DYNFLOW DFTC01C
0	50.	50.	50. (FH)	50. (FH)
10	55.	55.	55.	55.
20	60.	60.	60.	60.
30	65.	65.	65.	65.
40	70.	70.	70.	70.
50	75.	75.	75.	75.
60	80.	80.	80.	80.
70	85.	85.	85.	85.
80	90.	90.	90.	90.
90	95.	95.	95.	95.
100	100.	100.	100. (FH)	100. (FH)
FH - Fixed Head Node (For all 3 test cases, level 1 results are identical to level 2 results)				

Table B1-2

test case	DFTC01A	DFTC01B	DFTC01C
# of iterations	14	2	2
influx	1000.021 ft ³ /d	1000.013 ft ³ /d	499.994 ft ³ /d
outflux	1000.006 ft ³ /d	1000.012 ft ³ /d	499.994 ft ³ /d
total mass balance error	.0016%	1.0 E-4 %	0.0%

performance notes: Resulting heads for the three cases are identical to those for analytical solution (see Fig. B1-3). Flow rates for inflow (1000.021, 1000.013 and 499.994 ft³/d, respectively) and outflow (1000.021, 1000.012, and 499.994 ft³/d, respectively) match analytical results (1000.0, 1000.0, and 500.0 ft³/d, respectively) very well.

DFTC01B - Results for heads are the same as in test DFTC01A, but with a smaller mass balance error; grid and principal direction of hydraulic conductivity have been rotated 45 degrees about the z-axis.

DFTC01C - Results for heads are the same as in test DFTC01A, but with a zero mass balance error; elevations and principal direction of hydraulic conductivity are rotated 45 degrees about the x-axis; grid horizontal and vertical distances are reduced by 0.70711 to produce the required flow distance.

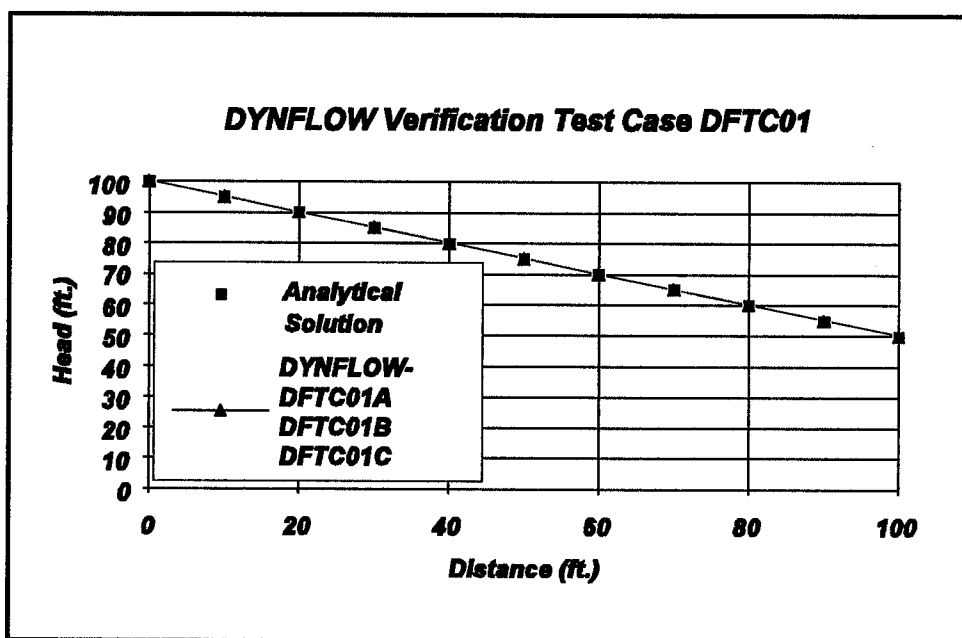


Figure B1-3. Head versus distance from origin (i.e., left boundary).

Command File DFTC01A.CFI

```

OUTPUT DFTC01A.OUT
TITLE
VERIFICATION CASE NO. 1A -- LINEAR STANDARD; RUN BY PVDH, BOULDER, CO.
LINEAR STEADY-STATE CONFINED 1D HORIZONTAL FLOW.
REF.: ANY GW FLOW TEXTBOOK.
GRID READ DFTC01A.GRF FORM
LEVEL 2.
FREE
PROP
1,10.,10.,10.,0.,0.,0.,0.,0.
!
```

route outputs (head, iteration, balance to file indicates next three lines are simulation title

reads formatted (.GRF) grid information specifies number of vertical model levels invokes specified flux (default) boundary conditions defines aquifer properties as below property set number, K_x (hydraulic conductivity in x direction), K_y, K_z storativity, specific yield,

Test Summary: DYNFLOW - Version 5.18 (September 1996)

```

!                               recharge/flux ,rotation about z, y axis (ccw)
ELEM  301                       selects element type computations, assigns property
!                               set numbers to elements; 3=three dimensional
!                               element computations, 01=property set number
ELEV  0.  LEVELSING             1   assigns elevations to node points (2 levels, 1 layer)
ELEV  20. LEVELSING             2   "
INIT  100.                       assigns initial heads to all nodes
INIT  50.  LEVELALL  NODE RANGE  31 33  assigns initial heads to specific nodes
FIX   90.  LEVELALL  NODE RANGE   1  3  invokes specified head boundary conditions at nodes
FIX   31.  LEVELALL  NODE RANGE  31 33  "
DT    0.                          assigns simulation time step duration
TOL   0.001                       assigns computation iteration tolerance
ITIN  90.                          sets max. number of inner loop iterations
ITER  30.                          sets max. number of outer loop iterations
ALPHA 1.5                          sets relaxation factor for internal iterations (D solver only)
NOPR                                     suppresses auto printing of tables
GOTIL 0.                             indicates steady state run
PRINT                                     prints the summary of condition tables
SUMMARY                                  prints property set assignment and mass balance tables
ERROR                                    prints table of errors or warnings
! REPLACE NEXT WITH COMMAND 'END' IF YOU LIKE
XCFI                                     returns control to interactive mode

```

Grid File DFTC01A.GRF

33	40						
1	0	10.0		18	50.0	0	
2	0	5.0		19	60.0	10.0	
3	0	0		20	60.0	5.0	
4	10.0	10.0		21	60.0	0	
5	10.0	5.0		22	70.0	10.0	
6	10.0	0		23	70.0	5.0	
7	20.0	10.0		24	70.0	0	
8	20.0	5.0		25	80.0	10.0	
9	20.0	0		26	80.0	5.0	
10	30.0	10.0		27	80.0	0	
11	30.0	5.0		28	90.0	10.0	
12	30.0	0		29	90.0	5.0	
13	40.0	10.0		30	90.0	0	
14	40.0	5.0		31	100.0	10.0	
15	40.0	0		32	100.0	5.0	
16	50.0	10.0		33	100.0	0	
17	50.0	5.0					
1	5	4	1	19	15	17	14
2	2	5	1	20	15	18	17
3	3	5	2				
4	3	6	5				
5	8	7	4				
6	5	8	4				
7	6	8	5				
8	6	9	8				
9	11	10	7				
10	8	11	7				
11	9	11	8				
12	9	12	11				
13	14	13	10				
14	11	14	10				
15	12	14	11				
16	12	15	14				
17	17	16	13				
18	14	17	13				

Test Summary: DYNFLOW - Version 5.18 (September 1996)

21	20	19	16
22	17	20	16
23	18	20	17
24	18	21	20
25	23	22	19
26	20	23	19
27	21	23	20
28	21	24	23
29	26	25	22
30	23	26	22
31	24	26	23
32	24	27	26
33	29	28	25
34	26	29	25
35	27	29	26
36	27	30	29
37	32	31	28
38	29	32	28
39	30	32	29
40	30	33	32

Command File DFTC01B.CFI

OUTPUT DFTC01B.OUT

TITLE

VERIFICATION CASE NO. 1B -- LINEAR HOR. ROTATED GRID; RUN BY PVDH, BOULDER, CO.
LINEAR STEADY-STATE CONFINED 1D HORIZONTAL FLOW.
REF.: ANY GW FLOW TEXTBOOK.

TEXT

THIS CASE IS THE SAME BASIC CASE AS CASE 1A EXCEPT
THAT THE GRID IS ROTATED 45 DEGREES AND THE
PRINCIPAL DIRECTION OF THE HYDRAULIC CONDUCTIVITY
IS LIKEWISE ROTATED IN THE HORIZONTAL PLANE. THIS
TESTS THE HORIZONTAL ANISOTROPY ALGORITHM - THE
CALCULATED HEADS AND FLUX SHOULD BE THE SAME AS CASE 1A.

ENDT

GRID READ DFTC01B.GRF FORM *grid is rotated 45 degrees*
LEVEL 2.
FREE
PROP
1,10.,0.,0.,0.,0.,0.,45.,0. *parameter 8 (rotates conductivity values ccw about the vertical axis)*
ELEM 301
ELEV 0. LEVELSING 1
ELEV 20. LEVELSING 2
INIT 100.
INIT 50. LEVELALL NODE RANGE 31 33
FIX LEVELALL NODE RANGE 1 3
FIX LEVELALL NODE RANGE 31 33
DT 0.
TOL 0.001
ALPHA 1.5
ITIN 90.
ITER 30.
NOPR
GOTI 0.
PRINT
SUMMARY
ERROR
XCFI

Grid File DFTC01B.GRF

33	40	5				
1	-7.071	7.071		1	5	4
2	-3.535	3.535		2	2	5
3	0.	0.		3	3	5
4	0.	14.141		4	3	6
5	3.535	10.606		5	8	7
6	7.071	7.071		6	5	8
7	7.071	21.212		7	6	8
8	10.606	17.677		8	6	9
9	14.141	14.141		9	11	10
10	14.141	28.283		10	8	11
11	17.677	24.749		11	9	11
12	21.212	21.212		12	9	12
13	21.212	35.355		13	14	13
14	24.749	31.820		14	11	14
15	28.282	28.282		15	12	14
16	28.282	42.426		16	12	15
17	31.820	38.891		17	17	16
18	35.355	35.355		18	14	17
19	35.355	49.497		19	15	17
20	38.891	45.962		20	15	18
21	42.426	42.426				
22	42.426	56.569				
23	45.962	53.033				
24	49.497	49.497				
25	49.497	63.640				
26	53.033	60.104				
27	56.569	56.569				
28	56.569	70.711				
29	60.104	67.175				
30	63.640	63.640				
31	63.640	77.782				
32	67.175	74.246				
33	70.711	70.711				
21	20	19	16			
22	17	20	16			
23	18	20	17			
24	18	21	20			
25	23	22	19			
26	20	23	19			
27	21	23	20			
28	21	24	23			
29	26	25	22			
30	23	26	22			
31	24	26	23			
32	24	27	26			
33	29	28	25			
34	26	29	25			
35	27	29	26			
36	27	30	29			
37	32	31	28			
38	29	32	28			
39	30	32	29			
40	30	33	32			

Command File DFTC01C.CFI

OUTPUT DFTC01C.OUT

TITLE

VERIFICATION CASE NO. 1C -- LINEAR VERT. ROTATED GRID; RUN BY PVDH, BOULDER, CO.
LINEAR STEADY-STATE CONFINED 1D HORIZONTAL FLOW.
REF.: ANY GW FLOW TEXTBOOK.

TEXT

THIS CASE IS THE SAME BASIC CASE AS CASE 1A EXCEPT THAT THE ELEVATIONS ARE ROTATED 45 DEGREES AND THE PRINCIPAL DIRECTION OF THE HYDRAULIC CONDUCTIVITY IS LIKEWISE ROTATED IN THE VERTICAL PLANE. THIS TESTS THE VERTICAL ANISOTROPY ALGORITHM - THE CALCULATED HEADS AND FLUX SHOULD BE THE SAME AS CASE 1A. NOTE THAT GRID IS EXPANDED BY ONE TIER OF ELEMENTS AND NODES AND THAT HORIZONTAL AND VERTICAL DISTANCES ARE REDUCED BY 0.70711 TO PRODUCE THE REQUIRED LENGTH OF 100 UNITS DIAGONALLY - ALSO THE THICKNESS NORMAL TO THE BEDDING IS NOW 10 UNITS AND THE HYDRAULIC CONDUCTIVITY IS 20 UNITS/TIME UNIT FOR THE SAME REASON.

ENDT

GRID READ DFTC01C.GRF FORM
LEVEL 2.

FREE

PROP

1,20.,0.,0.,0.,0.,0.,0.,-45.

parameter 9 rotates conductivity values ccw about the x-axis

ELEM 301

ELEV 0. LEVELSING 1 NODE RANGE 1 3 *grid elevations rotated 45 degrees*

ELEV -7.071 LEVELSING 1 NODE RANGE 4 6

ELEV -14.142 LEVELSING 1 NODE RANGE 7 9

ELEV -21.213 LEVELSING 1 NODE RANGE 10 12

ELEV -28.284 LEVELSING 1 NODE RANGE 13 15

ELEV -35.355 LEVELSING 1 NODE RANGE 16 18

ELEV -42.426 LEVELSING 1 NODE RANGE 19 21

ELEV -49.497 LEVELSING 1 NODE RANGE 22 24

ELEV -56.569 LEVELSING 1 NODE RANGE 25 27

ELEV -63.640 LEVELSING 1 NODE RANGE 28 30

ELEV -70.711 LEVELSING 1 NODE RANGE 31 33

ELEV -77.782 LEVELSING 1 NODE RANGE 34 36

ELEV LEVELSING 1 SAVE DFTC01C.ELV

ELEV LEVELSING 2 REXL DFTC01C.ELV

ELEV 7.071 LEVELSING 2 ADD

INIT 100.

INIT 50. LEVELALL NODE RANGE 31 36

FIX LEVELALL NODE RANGE 1 3

FIX LEVELSING 2 NODE RANGE 4 6

FIX LEVELALL NODE RANGE 34 36

FIX LEVELSING 1 NODE RANGE 31 33

DT 0.

TOL 0.001

ALPHA 1.5

ITIN 90.

ITER 30.

NOPR

GOTI 0.

PRINT

SUMMARY

ERROR

XCFI

Grid File DFTC01C.GRF

36	44						
1	0	10.0					
2	0	5.0		19	42.426	10.0	
3	0	0		20	42.426	5.0	
4	7.071	10.0		21	42.426	0	
5	7.071	5.0		22	49.497	10.0	
6	7.071	0		23	49.497	5.0	
7	14.142	10.0		24	49.497	0	
8	14.142	5.0		25	56.569	10.0	
9	14.142	0		26	56.569	5.0	
10	21.213	10.0		27	56.569	0	
11	21.213	5.0		28	63.640	10.0	
12	21.213	0		29	63.640	5.0	
13	28.284	10.0		30	63.640	0	
14	28.284	5.0		31	70.711	10.0	
15	28.284	0		32	70.711	5.0	
16	35.355	10.0		33	70.711	0	
17	35.355	5.0		34	77.782	10.0	
18	35.355	0		35	77.782	5.0	
				36	77.782	0	
1	5	4	1	43	33	35	32
2	2	5	1	44	33	36	35
3	3	5	2				
4	3	6	5				
5	8	7	4				
6	5	8	4				
7	6	8	5				
8	6	9	8				
9	11	10	7				
10	8	11	7				
11	9	11	8				
12	9	12	11				
13	14	13	10				
14	11	14	10				
15	12	14	11				
16	12	15	14				
17	17	16	13				
18	14	17	13				
19	15	17	14				
20	15	18	17				
21	20	19	16				
22	17	20	16				
23	18	20	17				
24	18	21	20				
25	23	22	19				
26	20	23	19				
27	21	23	20				
28	21	24	23				
29	26	25	22				
30	23	26	22				
31	24	26	23				
32	24	27	26				
33	29	28	25				
34	26	29	25				
35	27	29	26				
36	27	30	29				
37	32	31	28				
38	29	32	28				
39	30	32	29				
40	30	33	32				
41	35	34	31				
42	32	35	31				

Test 2: 1-D NON-LINEAR FLOW

test #: DFTC02 (DUPUIT)

input file: DFTC02.CFI

grid file: DFTC02.GRF

output file: DFTC02.OUT

manual reference: Section G-3

date of test execution: October 1996

title: DUPUIT: horizontal one-dimensional unconfined (non-linear) flow.

description: Steady-state horizontal flow between two fully penetrating canals with different surface potentials in an isotropic, homogeneous, unconfined aquifer with a horizontal impermeable base (see figure B2-1).

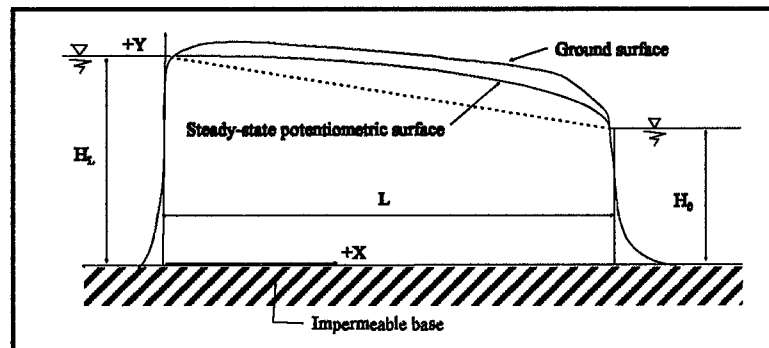


Fig. B2-1. Conceptual model for test case DFTC02.

tested functions: Unconfined potentiometric surface calculation (across multiple layers) and node point flux computations for case where principle direction of hydraulic conductivity is parallel with the flow direction; implementation of constant head boundary conditions.

model domain: Strip between two parallel drains with a length, and width of $L=100$ ft, and $w=10$ ft, respectively.

boundary conditions: Constant head at left boundary ($x=0$ ft) is $h_o=100$ ft; constant head at right boundary ($x=100$ ft) is $h_L=50$ ft; no-flow boundaries in y- and z-direction.

benchmark: Analytical solution for a nonlinear potentiometric surface between two fixed head boundaries in a unconfined aquifer under steady-state conditions as function of distance x from upper boundary; the solution is based on the Dupuit-Forcheimer assumption of fully horizontal flow (neglecting vertical flow in the unconfined aquifer, including near the outflow boundary):

$$h^2 = h_0^2 - \frac{x}{L}(h_0^2 - h_L^2)$$

and the discharge is calculated from:

$$Q = \frac{K}{2L}(h_0^2 - h_L^2) bw$$

where, Q = flow rate [L^3/T], K = hydraulic conductivity [L/T], H_L = fixed head at downstream boundary [L], H_0 = fixed head at upstream boundary [L], L = length of aquifer [L], b = aquifer thickness, and w = width of aquifer [L].

grid: Grid with five layer and six levels (0, 20, 40, 60, 80, and 200 ft); in plan view, the grid consists of 33 nodes in three rows, defining 40 elements (see figure B2-2); the horizontal definition of the grid is identical to the one used in the LINEAR test problem; heads are fixed below the top level to simulate unconfined conditions.

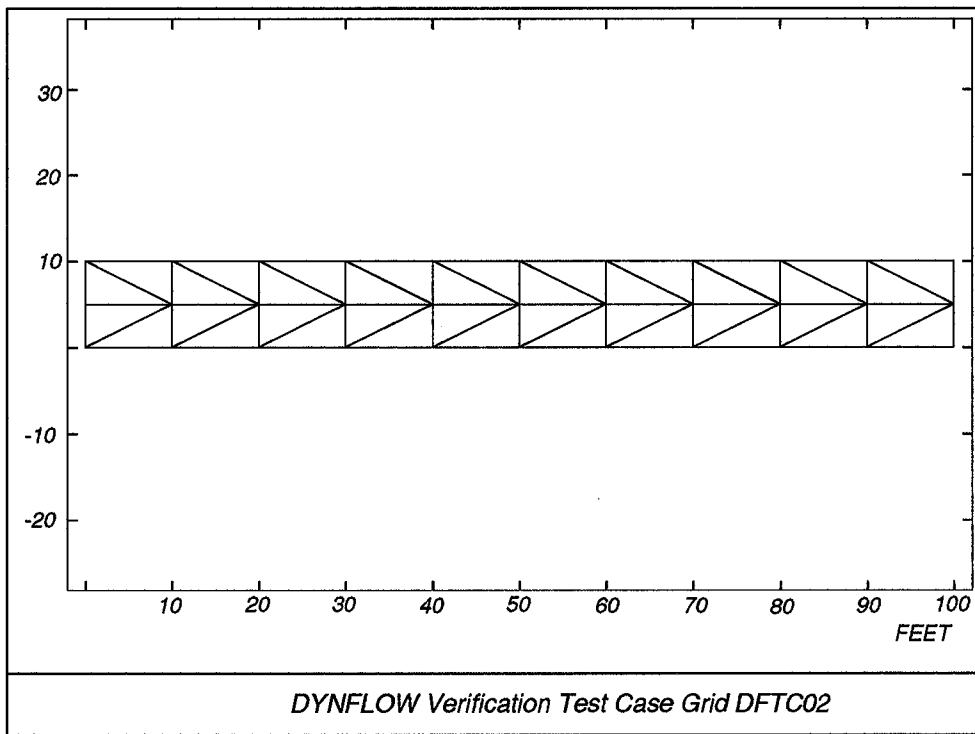


Figure B2-2.

initial conditions: n.a. (steady state)

time-stepping: n.a. (steady state)

Test Summary: DYNFLOW - Version 5.18 (September 1996)

system parameters: Hydraulic conductivity $K_x = K_y = K_z = 1.0$ ft/d; resulting $Q=375.0$ ft³/d.

control parameters: Tolerance = .001 ft; alpha = 1.5 (relaxation factor); max. # outer iterations = 10; max. # inner iterations = 90.

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Graphic plot of heads (see Fig. B2-3); tabular listing of heads (see Table B2-1); no statistical measures calculated.

Table B2-1

distance from downstream end (ft)	piezometric head (ft)						
	analytical solution	DYNFLOW DFTC02					
		level 1	level 2	level 3	level 4	level 5	level 6
0	50.	50. (FH)	50. (FH)	50. (FH)	50. (FH)	50. (FH)	50. (FH)
10	57.01.	56.97	56.98	57.03	57.09	57.09	57.09
20	63.25	63.21	63.22	63.25	63.31	63.33	63.33
30	68.92	68.89	68.89	68.92	68.96	68.99	68.99
40	74.16	74.13	74.14	74.16	74.19	74.23	74.23
50	79.06	79.03	79.03	79.05	79.08	79.12	79.12
60	83.67	83.64	83.64	83.66	83.68	83.72	83.73
70	88.03	88.01	88.01	88.02	88.04	88.07	88.09
80	92.20	92.17	92.17	92.18	92.20	92.23	92.25
90	96.18	96.15	96.16	96.17	96.18	96.20	96.22
100	100.	100. (FH)	100. (FH)	100. (FH)	100. (FH)	100. (FH)	100. (FH)
maximum difference with benchmark per level [ft]		.04	.03	.02	.08	.08	.08
FH - Fixed Head Node (The numbers listed for each level are the average of three values)							

Table B2-2

test case	DFTC02
max # of iterations	22 (level 1)
influx	375.150 ft ³ /d
outflux	375.165 ft ³ /d
total mass balance error	.0040%
max. head difference	.08 ft

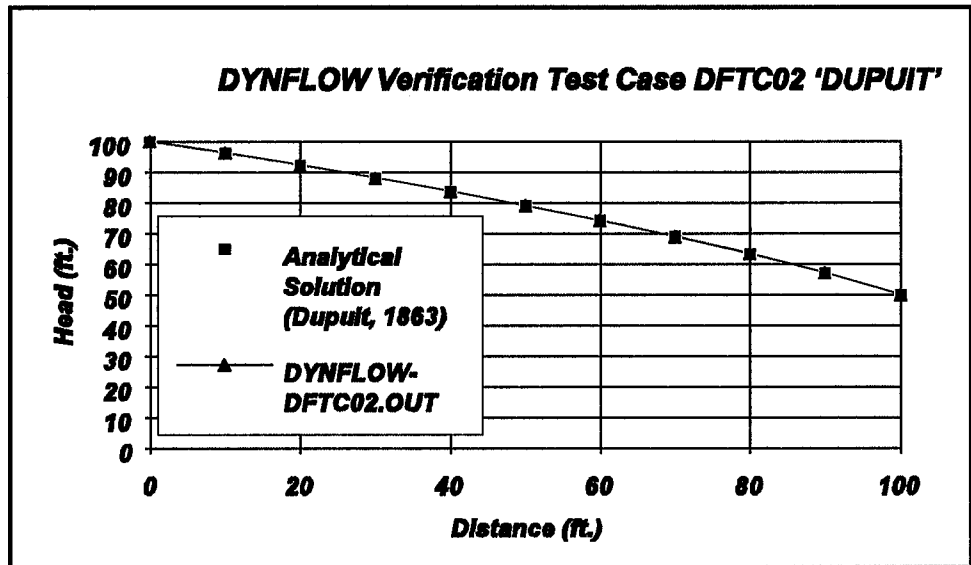


Figure B2-3. Heads versus distance from origin (i.e., left boundary).

performance notes: DFTC02 - Resulting heads differ slightly from the benchmark presented by the DYNFLOW authors (see Fig. B2-3). Computed flow rates for inflow and outflow differ about 0.04% from the analytical results (375.0 ft³/d).

Command File DFTC02.CFI

OUTPUT DFTC02.OUT

!

TITLE

VERIFICATION CASE NO. 2 -- DUPUIT; RUN BY PVDH, BOULDER, CO.

DUPUIT STEADY-STATE UNCONFINED 1D HORIZ FLOW.

REF.: FREEZE AND CHERRY, "GROUNDWATER," PAGES 188-189.

!

TEXT

$$H^2 = H_0^2 - (x / L) (H_0^2 - H_1^2)$$

$$Q = (k / (2 L)) (H_0^2 - H_1^2) * w$$

WHERE:

H = HEAD AT ANY POINT x.

x = DISTANCE FROM POINT 0 TO POINT L.

0 = POINT MARKER FOR "LEFT" END OF AQUIFER.

1 = POINT MARKER FOR "RIGHT" END OF AQUIFER.

H0 = HEAD AT POINT 0.

L = LENGTH OF AQUIFER.

H1 = HEAD AT POINT 1.

k = HYDRAULIC CONDUCTIVITY.

^2 = SQUARE.

LET:

H0 = 50.0

H1 = 100.0

L = 100.0

k = 1.0

w = 10.0

SOLUTION:

$$H^2 = 2500 + 75 x$$

x	H
10.	57.01
20.	63.25
30.	68.92
40.	74.16
50.	79.06
60.	83.67
70.	88.03
80.	92.20
90.	96.18

$$Q = 375.0$$

ENDT

!

GRID READ DFTC02.GRF FORM

LEVEL 6.

FREE

PROP

1,1.,1.,1.,0.,0.,0.,0.,0.

PROP

2,1.,1.,1.,0.,0.,0.

ELEM 301. &

LAYERRANGE 1 3

ELEM 302. LAYERRANGE 4 6

ELEV 0. LEVELSING 1

ELEV 20. LEVELSING 2

ELEV 40. LEVELSING 3

Test Summary: DYNFLOW - Version 5.18 (September 1996)

```

ELEV 60.    LEVELSING  4
ELEV 80.    LEVELSING  5
ELEV 200.   LEVELSING  6
INIT 100.
INIT 50.    LEVELALL      NODE RANGE  1  3    Initial conditions set below top level to simulate
                                           unconfined conditions
FIX        LEVELALL      NODE RANGE  1  3
FIX        LEVELALL      NODE RANGE 31 33
DT 0.
ITIN 90.
ALPHA 1.5
ITER 30.
TOL .001
NOPR
GOTIL 0.
PRINT
SUMMARY
ERROR

```

```

C00000000111111111222222222223333333333344444444445555555555666666666677777777778
C2345678901234567890123456789012345678901234567890123456789012345678901234567890

```

```

HEAD        LEVELSING  1    NODE &
 2, 5, 8, 11, 14, 17, 20, 23, 26, 29, 32, &
SAVE DFTC02.CHK NOHD
FLFF SUM
SAVE DFTC02.SAV
END

```

Grid File DFTC02.GRF

33	40				
1	0	10.0	18	50.0	0
2	0	5.0	19	60.0	10.0
3	0	0	20	60.0	5.0
4	10.0	10.0	21	60.0	0
5	10.0	5.0	22	70.0	10.0
6	10.0	0	23	70.0	5.0
7	20.0	10.0	24	70.0	0
8	20.0	5.0	25	80.0	10.0
9	20.0	0	26	80.0	5.0
10	30.0	10.0	27	80.0	0
11	30.0	5.0	28	90.0	10.0
12	30.0	0	29	90.0	5.0
13	40.0	10.0	30	90.0	0
14	40.0	5.0	31	100.0	10.0
15	40.0	0	32	100.0	5.0
16	50.0	10.0	33	100.0	0
17	50.0	5.0			

1	5	4	1
2	2	5	1
3	3	5	2
4	3	6	5
5	8	7	4
6	5	8	4
7	6	8	5
8	6	9	8
9	11	10	7
10	8	11	7
11	9	11	8
12	9	12	11

Test Summary: DYNFLOW - Version 5.18 (September 1996)

13	14	13	10
14	11	14	10
15	12	14	11
16	12	15	14
17	17	16	13
18	14	17	13
19	15	17	14
20	15	18	17
21	20	19	16
22	17	20	16
23	18	20	17
24	18	21	20
25	23	22	19
26	20	23	19
27	21	23	20
28	21	24	23
29	26	25	22
30	23	26	22
31	24	26	23
32	24	27	26
33	29	28	25
34	26	29	25
35	27	29	26
36	27	30	29
37	32	31	28
38	29	32	28
39	30	32	29
40	30	33	32

Test 3: TANK/POND DRAINAGE

tests #: DFTC03A, DFTC03B, DFTC03C

command files: DFTC03A.CFI, DFTC03B.CFI, DFTC03C.CFI

grid files: DFTC03A.GRF, DFTC03B.GRF, DFTC03C.GRF

output files: DFTC03A.OUT, DFTC03B.OUT, DFTC03C.OUT

manual reference: Section G-4

date of test execution: October 1996

title: POND: vertical one-dimensional transient flow

description: A vertical tank or pond, initially full, is drained over time causing vertical flow in an isotropic, homogeneous, unconfined porous medium (see figure B3-1).

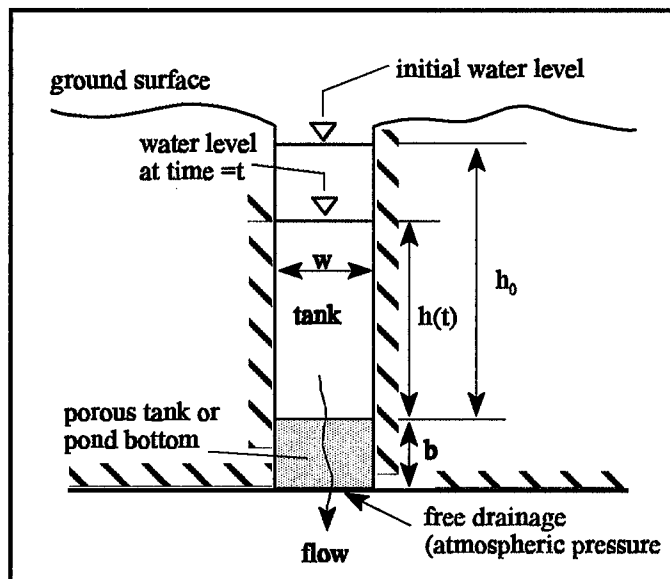


Figure B3-1. Conceptual model for test case DFTC03.

tested functions: Vertical flow algorithms and unconfined potentiometric surface calculation under transient conditions (DFTC03A); functioning of 1D elements (DFTC03B), and operation of pond elements (DFTC03C).

model domain: Porous strip of constant thickness between two parallel, vertical, impermeable walls with a length (perpendicular to drawing in Figure B3.1), width and thickness of $L=10$ ft, $w=10$ ft, and $b=10$ ft, respectively.

boundary conditions: Constant head at lower boundary ($z=0$ ft) is $h_0=.001$ ft representing atmospheric pressure; free surface at top boundary; no-flow boundaries in x- and y-direction; initial condition is set at $H_0 = 80$ ft.

benchmark: Analytical solution for head at the inflow point as function of time (derived by DYNFLOW authors):

$$h(t) = h_0 e^{-\frac{k}{b} t}$$

and the discharge is calculated from:

$$Q = K \frac{h(t)}{b} Lw$$

where, Q = flow rate [L^3/T], K = hydraulic conductivity [L/T], L = length of porous section in y-direction [L], b = thickness of porous section, and w = width of porous section (in x-direction) [L].

grid: DFTC03A: grid with eight layers and nine levels, where the first 7 layers are 10 ft thick and the 8th layer (top layer) is 11 ft; in plan view, the grid consists of 4 nodes and 2 elements (see figure B3-2); the boundary condition at the top is represented by a series of cells with a decreasing water table.

DFTC03B: grid with eight layers and nine levels, where the first 7 layers are 10 ft thick and the 8th layer (top layer) is 11 ft; in plan view, the grid consists of 1 onedimensional element; the boundary condition at the top is represented by a series of cells with a decreasing water table.

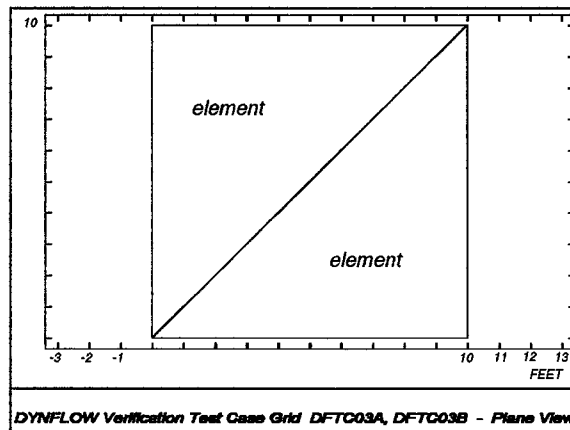


Figure B3-2.

DFTC03C: grid with one layer and two levels and a layer thickness of 10 ft; top boundary is represented by pond element; in plan view, the grid consists of 4 nodes and 2 elements (see figure B3-2).

initial conditions: $h=80$ ft

time-stepping: 10 time periods of 10 days; time steps of 1 day; to incorporate the initial condition, the first time period of 10 days has been divided in a large number of time steps of increasing size.

system parameters: Hydraulic conductivity $K_x = K_y = K_z = 0.1$ ft/d; specific storativity = 0.0 and specific yield = 1.0.

control parameters: Tolerance = .0001 ft (DFTC03A/B), .001 (DFTC03C); alpha = 0.5 (relaxation factor); max. # outer iterations = 30; max. # inner iterations = 50

solver: ICCG (DFTC03A/B); GAUSS (DFTC03C); using ICCG solver in test case DFTC03C resulted in fatal error message: "ROUTINE ITCGIT - Q IS NOT POSITIVE DEFINITE"

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Graphic plot of heads (see Fig. B3-3); tabular listing of heads (see Table B3-1, B3-2 and B3-3); no statistical measures calculated.

Table B3-1

time (days)	piezometric head (ft)										
	analytical solution	inner (outer) iterations	DYNFLOW DFTC03A								
			1 ¹	2	3	4	5	6	7	8	9
10	72.39	7(2)	.0	72.16	72.20	72.23	72.27	72.30	72.34	72.38	72.41
20	65.50	5(2)	.0	65.31	65.35	65.38	65.41	65.44	65.48	65.51	65.51
30	59.26	5(2)	.0	59.12	59.14	59.17	59.20	59.23	59.26	59.26	59.26
40	53.63	5(2)	.0	53.50	53.53	53.56	53.58	53.61	53.64	53.64	53.64
50	48.52	5(2)	.0	48.45	48.47	48.49	48.52	48.52	48.52	48.52	48.52
60	43.90	5(2)	.0	43.82	43.84	43.87	43.89	43.91	43.91	43.91	43.91
70	39.73	5(3)	.0	39.66	39.68	39.70	39.72	39.72	39.72	39.72	39.72
80	35.95	5(2)	.0	35.89	35.91	35.92	35.94	35.94	35.94	35.94	35.94
90	32.53	5(3)	.0	32.48	32.49	32.51	32.53	32.53	32.53	32.53	32.53
100	29.43	4(7)	.0	29.39	29.40	29.42	29.42	29.42	29.42	29.42	29.42
maximum difference with benchmark per level [ft]			.0	.23	.19	.16	.12	.09	.05	.01	.02
For each time, all values of the four nodes at each level are identical; thus value listed is representative for particular level. 1) fixed head level											

Table B3-2

time (days)	piezometric head (ft)		
	analytical solution	DYNFLOW ¹⁾	
		DFTC03B	DFTC03C
10	72.39	72.38 - 72.39	72.44
20	65.50	65.50	65.60
30	59.26	59.26-59.27	59.40
40	53.63	53.63	53.78
50	48.52	48.53	48.70
60	43.90	43.92	44.09
70	39.73	39.75	39.92
80	35.95	35.96	36.14
90	32.53	32.54	32.72
100	29.43	29.45	29.61
1) For each time, the range of values is given			

Table B3-3.

test case	Benchmark	DFTC03A	DFTC03B ¹⁾	DFTC03C ¹⁾
influx (at 100 days)	29.43 ft ³ /d	29.464 ft ³ /d	.297 ft ³ /d	.297 ft ³ /d
outflux (at 100 days)	29.43 ft ³ /d	29.454 ft ³ /d	.299 ft ³ /d	.296 ft ³ /d
total mass balance error (at 100 days)		.034%	.77%	.29%
1) Because case 3B is one-dimensional, multiply flux with cross-sectional area of 100 ft ² to compare flux for case 3B with cases 3A and 3C.				

performance notes: DFTC03A - Although the resulting heads differ slightly from the benchmark presented by the DYNFLOW authors, especially in the early times, they are small and not visible in a graphic representation (see Fig. B3-3). Computed flow rates for inflow and outflow differ about 0.12% from the analytical results.

DFTC03B - The one-dimensional element approach is highly accurate with respect to heads, but has a larger mass balance error than the other cases, as well as a greater difference in flow rates with the benchmark (1%).

DFTC03C - There are some convergence problems using the pond element. The PCG solver did not run (see comment above) and the Gauss solver did not converge for a tolerance = 0.0001 as was used in the other cases; also, the 'SHOW' command for the POND function had to be commented out in the configuration file as this caused program abortion with an error message related to output format problems. The resulting heads differ slightly from the benchmark presented by the DYNFLOW authors (up to 0.6%), but are not large enough to be visible in a graphic representation (see Fig. B3-3). As is the case with DFTC03B, this case shows about 1% difference in external flux compared with the benchmark.

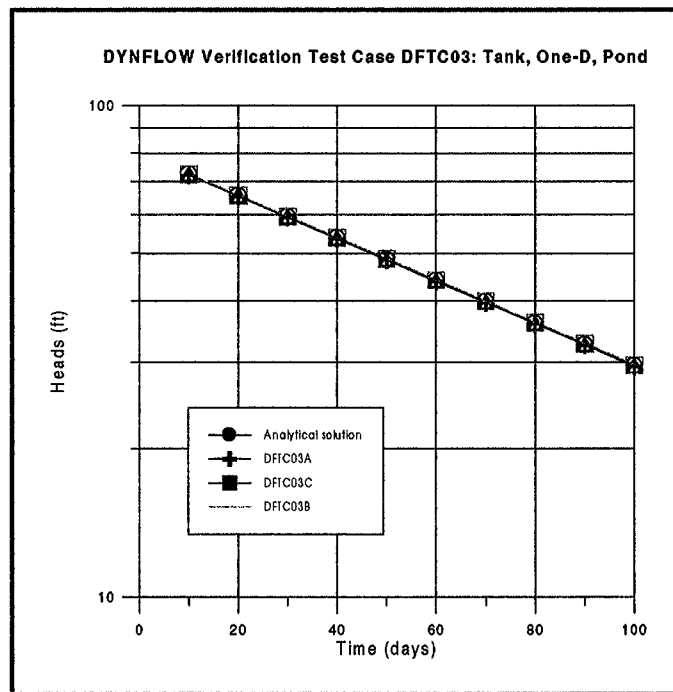


Figure B3-3.

Command File DFTC03A.CFI

OUTPUT DFTC03A.OUT
!
TITL
VERIFICATION CASE NO. 3 -- TANK; RUN BY PVDH, BOULDER, CO.
TANK TRANSIENT UNCONFINED 1D VERTICAL FLOW.
REF.: ANY DIFFERENTIAL CALCULUS BOOK.
TEXT

$$H(t) = H_0 \exp(- (k / b) t)$$

WHERE:

H(t) = HEAD
t = TIME
H0 = HEAD AT STARTING TIME
EXP = NATURAL EXPONENTIAL FUNCTION
k = VERTICAL HYDRAULIC CONDUCTIVITY
b = THICKNESS OF LOWER k (LEAKY) LAYER

LET:

H0 = 80.0
k = 0.1
b = 10.0

SOLUTION:

t	H(t)
10.	72.387
20.	65.498
30.	59.265
40.	53.626
50.	48.522
60.	43.905
70.	39.727
80.	35.946
90.	32.526
100.	29.430

ENDT
!
GRID READ DFTC03A.GRF FORM
LEVEL 9.
FREE
ELEV 0. LEVELSING 1
ELEV 10. LEVELSING 2
ELEV 20. LEVELSING 3
ELEV 30. LEVELSING 4
ELEV 40. LEVELSING 5
ELEV 50. LEVELSING 6
ELEV 60. LEVELSING 7
ELEV 70. LEVELSING 8
ELEV 81. LEVELSING 9
PROP
1, 1., 1., 0.1, 0., 1.0, 0., 0., 0.
ELEM 301. LAYERSING 1
PROP
2, 1., 1., 10000., 0., 1.0, 0.
ELEM 302. LAYERRANGE 2 8
INIT 80.
INIT 0.001 LEVELSING 1
FIX LEVELSING 1
DT .01
ITIN 50.
ALPHA 0.5

Test Summary: DYNFLOW - Version 5.18 (September 1996)

```

ITER 30.
TOL 0.0001
NOPR
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL .01
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
DT .1
GOTIL .1
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL .5
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
PRINT
DT .1
GOTIL 1.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 2.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
DT 1.
GOTIL 5.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 10.
PRINT
C0000000011111111112222222223333333334444444445555555556666666667777777778
C2345678901234567890123456789012345678901234567890123456789012345678901234567890
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 20.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 30.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 40.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 50.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 60.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 70.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 80.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 90.
PRINT
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 100.
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
PRINT
SUMM
ERROR
XCFI

```

Grid File DFTC03A.GRF

4	2		
1	0.0	0.0	
2	10.0	0.0	
3	10.0	10.0	
4	0.0	10.0	
1	1	2	3
2	1	3	4

Command File DFTC03B.CFI

OUTPUT DFTC03B.OUT

!
TITL
VERIFICATION CASE NO. 3B -- TANK, 1-DL ELEMENTS; RUN BY PVDH, BOULDER, CO.
TANK TRANSIENT UNCONFINED 1D VERTICAL FLOW.
REF.: ANY DIFFERENTIAL CALCULUS BOOK.
TEXT

$$H(t) = H_0 \text{ EXP}(- (k / b) t)$$

WHERE:

H(t) = HEAD
t = TIME
H0 = HEAD AT STARTING TIME
EXP = NATURAL EXPONENTIAL FUNCTION
k = VERTICAL HYDRAULIC CONDUCTIVITY
b = THICKNESS OF LOWER k (LEAKY) LAYER

LET:

H0 = 80.0
k = 0.1
b = 10.0

SOLUTION:

t	H(t)
10.	72.387
20.	65.498
30.	59.265
40.	53.626
50.	48.522
60.	43.905
70.	39.727
80.	35.946
90.	32.526
100.	29.430

ENDT

!
GRID READ DFTC03B.GRF FORM
LEVEL 9.
FREE
ELEV 0. LEVELSING 1
ELEV 10. LEVELSING 2
ELEV 20. LEVELSING 3
ELEV 30. LEVELSING 4
ELEV 40. LEVELSING 5

Test Summary: DYNFLOW - Version 5.18 (September 1996)

```

ELEV 50.      LEVELSING      6
ELEV 60.      LEVELSING      7
ELEV 70.      LEVELSING      8
ELEV 81.      LEVELSING      9
PROP
11,0.1,0.1,0.1,0.,1.0,0.,0.,0.
ELEM 100.     LAYERSING      1
ONED  111.    LEVELRANGE     1    2NODE SING    1    1
PROP
12,10000.,10000.,10000.,0.,1.0,0.,0.,0.
ELEM 100.     LAYERRANGE     2    8
ONED  112.    LEVELRANGE     2    3NODE SING    1    1
ONED  112.    LEVELRANGE     3    4NODE SING    1    1
ONED  112.    LEVELRANGE     4    5NODE SING    1    1
ONED  112.    LEVELRANGE     5    6NODE SING    1    1
ONED  112.    LEVELRANGE     6    7NODE SING    1    1
ONED  112.    LEVELRANGE     7    8NODE SING    1    1
ONED  112.    LEVELRANGE     8    9NODE SING    1    1
INIT  80.
INIT  0.01    LEVELSING      1
FIX   1        LEVELSING      1
DT    .01
ITIN  50.
ALPHA 0.5
ITER  30.
TOL  0.0001
NOPR
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL .01
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
DT    .02
GOTIL .1
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL .5
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
DT    .1
GOTIL 1.
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 2.
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
DT    1.
GOTIL 5.
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 10.
PRINT
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 20.
PRINT
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 30.
PRINT
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 40.
PRINT
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 50.
PRINT
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 60.
PRINT
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 70.
PRINT
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 80.
PRINT
HEAD          LEVELSING      9    NODE SING    1    SAVE DFTC3.TMP NOHD
GOTIL 90.
PRINT

```

Test Summary: DYNFLOW - Version 5.18 (September 1996)

```
C00000000111111111222222222233333333334444444445555555556666666667777777778
C2345678901234567890123456789012345678901234567890123456789012345678901234567890
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 100.
HEAD          LEVELSING      9      NODE SING      1      SAVE DFTC3.TMP NOHD
PRIN
SUMM
ERRO
XCFI
```

Grid File DFTC03B.GRF

```
1      1
1      0.0      0.0
1      1      1      0
```

Command File DFTC03C.CFI

```
OUTPUT DFTC03C.OUT
!
TITLE
VERIFICATION CASE NO. 3C -- POND.
POND - SAME AS TANK, BUT USING POND ELEMENTS INSTEAD; RUN BY PVDH, BOULDER, CO.
REF.: ANY DIFFERENTIAL CALCULUS BOOK.
TEXT
```

$$H(t) = H_0 \exp(- (k / b) t)$$

WHERE:

H(t) = HEAD
t = TIME
H0 = HEAD AT STARTING TIME
EXP = NATURAL EXPONENTIAL FUNCTION
k = VERTICAL HYDRAULIC CONDUCTIVITY
b = THICKNESS OF LOWER k (LEAKY) LAYER

LET:

H0 = 80.0
k = 0.1
b = 10.0

SOLUTION:

t	H(t)
10.	72.387
20.	65.498
30.	59.265
40.	53.626
50.	48.522
60.	43.905
70.	39.727
80.	35.946
90.	32.526
100.	29.430

```
ENDT
!
GRID READ DFTC03C.GRF FORM
LEVEL 2.
FREE
```

Test Summary: DYNFLOW - Version 5.18 (September 1996)

```

ELEV 0.      LEVELSING 1
ELEV 10.     LEVELSING 2
PROP
1,100.,100.,0.1,0.,1.0,0.,0.,0.
ELEM 301.
INIT 0.001   LEVELSING 1
INIT 80.     LEVELSING 2
FIX        LEVELSING 1
DT 10.
!
POND
NUMB
1
DATE
8,14,76,
CURV
3
10.,50.,100.,
0.,1.,1.,
0.,40.,90.,
PNOD
1,2,3,4,0
SPEC
70.0,0.0
!SHOW
ENDP
!
ITIN 50.
ALPHA 0.5
ITER 30.
TOL 0.001
NOPR
DT 0.01
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTI 0.01
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
DT .1
GOTIL .1
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTIL .5
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
DT .1
GOTIL 1.
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTIL 2.
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
DT 1.
GOTIL 5.
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTIL 10.
PRINT
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTIL 20.
PRINT
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTIL 30.
PRINT
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTIL 40.
PRINT
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTIL 50.
PRINT
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTIL 60.
PRINT
HEAD          LEVELSING 2      NODE SING 1      SAVE DFTC3.TMP NOHD
GOTIL 70.
PRINT

```

Test Summary: DYNFLOW - Version 5.18 (September 1996)

```
HEAD          LEVELSING      2      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 80.
PRINT
HEAD          LEVELSING      2      NODE SING      1      SAVE DFTC3.TMP NOHD
C0000000011111111122222222233333333334444444445555555556666666667777777778
C234567890123456789012345678901234567890123456789012345678901234567890
GOTIL 90.
PRINT
HEAD          LEVELSING      2      NODE SING      1      SAVE DFTC3.TMP NOHD
GOTIL 100.
HEAD          LEVELSING      2      NODE SING      1      SAVE DFTC3.TMP NOHD
PRIN
SUMM
ERRO
XCFI
```

Grid File DFTC03C.GRF

```
4          2
1          0.0      0.0
2          1.0      0.0
3          1.0      1.0
4          0.0      1.0
1          1          2          3
2          1          3          4
```

Test 4: THEIS CURVES

test #: DFTC04

command file: DFTC04.CFI

grid file: DFTC04.GRF

output file: DFTC04.OUT

manual reference: Section G-5

date of test execution: October 1996

title: THEIS: radial confined transient flow.

description: Transient flow towards a fully penetrating pumping well with a constant discharge rate in an isotropic, homogeneous, confined porous medium of infinite extent and constant thickness (see figure B4-1).

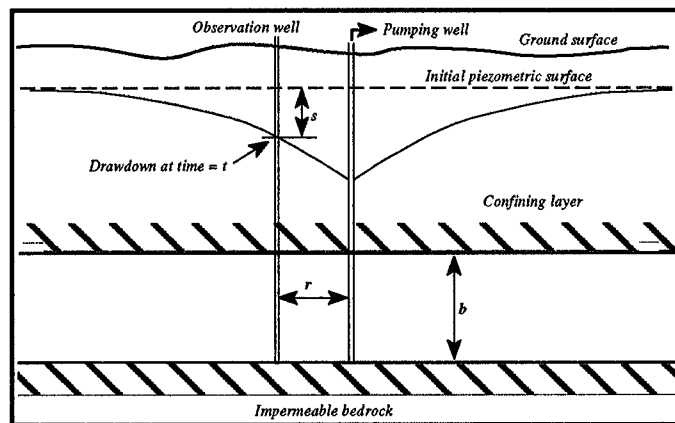


Figure B4-1.

tested functions: Transient horizontal flow under confined conditions; discharging well; lateral fixed head boundary condition.

model domain: Analytical solution: infinite horizontal extent, aquifer thickness $b = 20$ ft.

boundary conditions: Fixed head at outer radial boundary ($R=11,900$ ft) is $h_R = 100$ ft; well discharge rate $Q = 160,000$ ft³/d is distributed as a specified flux boundary condition in the nodes located about 40 ft from the center of the quadrant (i.e., location of well in analytical solution); remaining boundary nodes are specified as no-flow boundaries.

benchmark: Theis analytical solution for drawdown with given transmissivity, storativity and pumping rate:

$$s = \frac{Q}{4\pi T} W(u)$$

where

$$u = \frac{r^2 S}{tT}$$

where, Q = flow rate [L^3/T], T = transmissivity [L^2/T] = $k_h b$, k_h = horizontal hydraulic conductivity of aquifer [L/T], b = thickness of aquifer [L], S = storativity [fraction], r = distance from well [L], t = time since start of pumping [T], s = drawdown with respect to prepumping horizontal piezometric surface [L], and W = the Theis Well Function [dimensionless]. Calculation are made for observation wells at $r = 200, 650$ and 1000 ft from the well, respectively.

grid: Single layer, 1/2-quadrant (45°) grid with two levels (see Figure B4-2); in plan view, the grid consists of 25 nodes defining 30 elements.

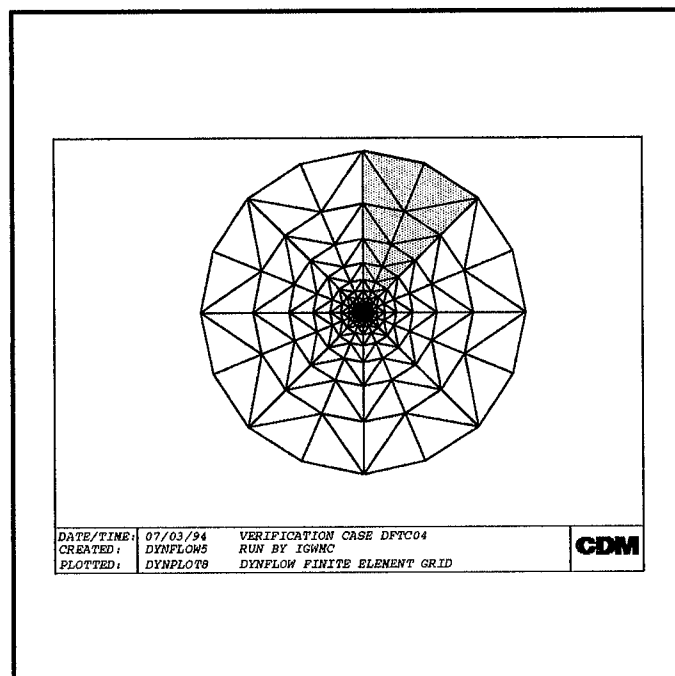


Figure B4-2.

initial conditions: $h=100$ ft

time-stepping: Preset time steps of 0.5 days, which is greater than the final time of computation; actual time step set such that each comparison time is a single time step from previous comparison time.

system parameters: Hydraulic conductivity $k_x = k_y = K_h = 500$ ft/d, $k_z = 1$ ft/d; specific storativity = 0.00005 ft⁻¹; aquifer specific yield = 0.10.

control parameters: Tolerance = .0001 ft; alpha = 1.5 (relaxation factor); acce = 1.5; max. # outer iterations = 30; max. # inner iterations = 90

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Graphic plot of heads (see Fig. B4-3); tabular listing of heads (see Table B4-1, and B4-2; no statistical measures calculated.

Table B4-1.

time (days)	# iterations	drawdown (ft)					
		r = 200 ft (node 4)		r = 650 ft (node 9)		r = 1000 ft (node 12)	
		Theis	DF ¹⁾	Theis	DF ¹⁾	Theis	DF ¹⁾
$t_1 = 0.005$	9	1.6	1.60-1.43	0.05	.09- .10	--	.01
$t_2 = 0.0125$	9	2.65	2.92-2.75	0.36	.33	0.06	.06
$t_3 = 0.02375$	10	3.4	3.52-3.38	0.78	.79- .80	0.25	.22
$t_4 = 0.040625$	10	4.0	4.23-4.00	1.21	1.27-1.28	0.57	.53
$t_5 = 0.0659375$	10	4.7	4.73-4.60	1.7	1.75	0.92	.89- .90
$t_6 = 0.1$	11	5.2	5.28-5.12	2.2	2.19	1.3	1.26

1) For each time, the range of values from different levels is given, if applicable

Table B4-2. Global fluxes in ft³/d

time step >	t ₁	t ₂	t ₃	t ₄	t ₅	t ₆
influx including storage	20000.06	20229.04	20023.72	20407.38	20086.71	20567.87
outflux	20005.44	20227.76	20030.75	20407.43	20090.15	20568.34
total mass balance error	.03%	.01%	.04%	.0003%	.02%	.002%

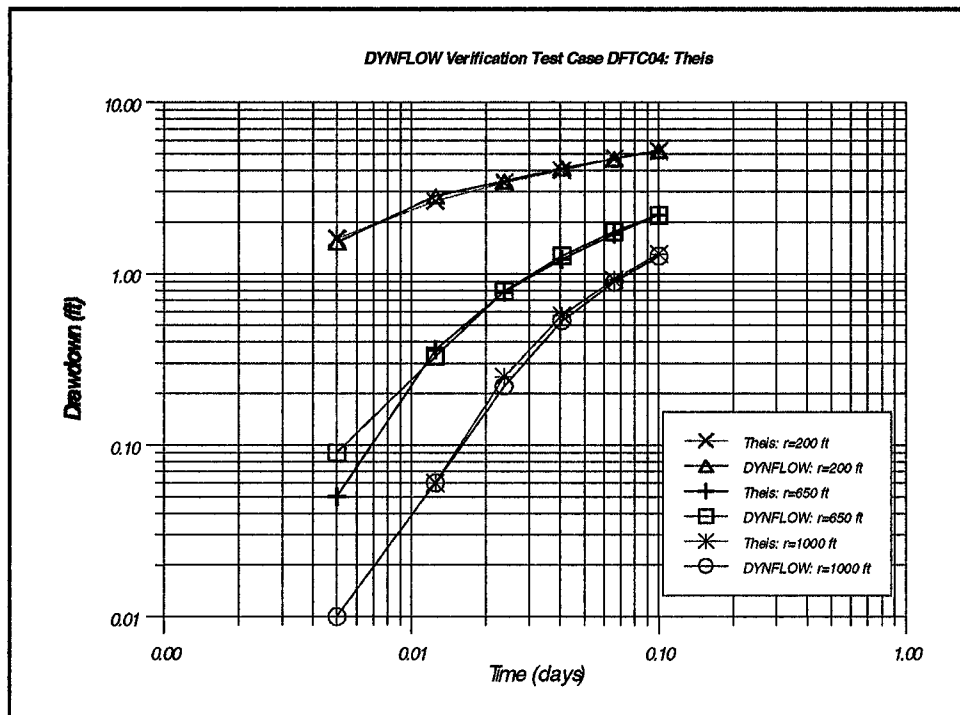


Figure B4-3.

performance notes: Although the resulting heads differ slightly from the benchmark presented by the DYNFLOW authors, especially in the early times, they are reasonably small. Mass balance error is very small for each of the time steps, although the flux varies slightly (note that pumping is constant).

Command File DFTC04.CFI

OUTPUT DFTC04.OUT

!

TITLE

VERIFICATION CASE NO. 4 -- THEIS; RUN BY PVDH, BOULDER, CO.
 THEIS TRANSIENT PHREATIC AND CONFINED (MIXED) 2D
 REF.: LOHMAN, "GROUND-WATER HYDRAULICS," USGS PROF PAPER 708, PAGES 30-32.
 TEXT

LET:

Kh = HORIZ HYDRAULIC CONDUCTIVITY
 = 500.
 Kv = VERT HYDRAULIC CONDUCTIVITY
 = Kh
 b = AQUIFER THICKNESS
 = 20.
 Ss = STORATIVITY
 = 0.00005
 Sy = SPECIFIC YIELD
 = 0.10
 Q = WELL PUMPING RATE
 = 160,000.

SOLUTION:

OBTAIN IT BY USING THE PLATES GIVEN IN LOHMAN'S PAPER,
 WHICH FOR THREE POINTS IN AQUIFER IS AS FOLLOWS (APPX.):

NODE POINT	4	9	12
DISTANCE	200.	650.	1000.

TIME	DRAWDOWN		
	4	9	12
.005	1.6	0.05	N/A
.0125	2.65	0.36	0.06
.02375	3.4	0.78	0.25
.040625	4.0	1.21	0.57
.0659375	4.7	1.7	0.92
.1	5.2	2.2	1.3

ENDT

!

GRID READ DFTC04.GRF FORM

LEVEL 2.

FREE

ELEV 0. LEVELSING 1

ELEV 20. LEVELSING 2

PROP

1,500.,500.,1.,0.00005,0.10,0.,0.,0.

ELEM 301.

INIT 100.

FIX LEVELALL NODE RANGE 23 25

FLUX -5000. LEVELALL NODE RANGE 2 3

DT 0.005

TOL .0001

ACCE 1.5

PRAL

ALPHA 1.5

ITIN 90.

C000000001111111122222222233333333334444444445555555556666666667777777778

C234567890123456789012345678901234567890123456789012345678901234567890

ITER 30.

Test Summary: DYNFLOW - Version 5.18 (September 1996)

HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .005						
print						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .0125						
print						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .02375						
print						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .040625						
print						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .0659375						
print						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .1						
print						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
XCFI						

Grid File DFTC04.GRF

25	30				
1	-100.	0.			
2	-92.388	38.2683			
3	-92.388	-38.2683			
4	-200.	0.			
5	-323.3578	133.9392			
6	-350.	0.			
7	-323.3578	-133.9392			
8	-600.5217	248.7442			
9	-650.	0.			
10	-600.5217	-248.7442			
11	-923.88	382.683			
12	-1000.	0.			
13	-923.88	-382.683			
			14	-1478.207	612.2935
			15	-1600.	0.
			16	-1478.207	-612.2935
			17	-2771.639	1148.0503
			18	-3000.	0.
			19	-2771.639	-1148.050
			20	-5543.227	2296.1006
			21	-6000.	0.
			22	-5543.227	-2296.101
			23	-11086.55	4592.2012
			24	-12000.	0.
			25	-11086.55	-4592.201

1	1	2	4	16	12	16	13
2	1	4	3	17	14	15	12
3	2	5	4	18	12	15	16
4	3	4	7	19	14	17	15
5	5	6	4	20	15	19	16
6	4	6	7	21	17	18	15
7	5	8	6	22	15	18	19
8	6	10	7	23	17	20	18
9	8	9	6	24	18	22	19
10	6	9	10	25	20	21	18
11	8	11	9	26	18	21	22
12	9	13	10	27	20	23	21
13	11	12	9	28	21	25	22
14	9	12	13	29	23	24	21
15	11	14	12	30	21	24	25

Test 5: HANTUSH SOLUTION

test #: DFTC05

command file: DFTC05.CFI

grid file: DFTC05.GRF

output file: DFTC05.OUT

date of test execution: October 1996

title: HANTUSH: radial leaky-confined transient flow

description: Transient flow towards a fully penetrating pumping well with a constant discharge rate in an isotropic, homogeneous, leaky-confined porous medium of infinite extent and constant thickness (see figure B5-1).

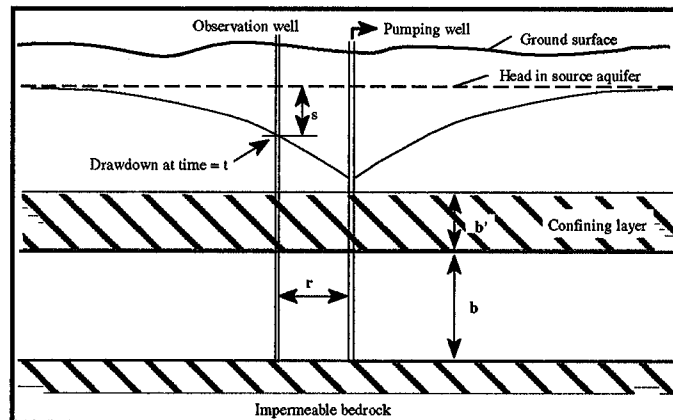


Figure B5-1.

tested functions: Transient horizontal and vertical flow under confined conditions; lateral and top fixed head boundary condition; discharging well.

model domain: Analytical solution: infinite horizontal extent, aquifer thickness $b = 20$ ft, aquitard thickness $b' = 10$ ft.

boundary conditions: Fixed head at outer radial boundary ($R=11,900$ ft) is $h_R = 100$ ft; well discharge rate $Q = 160,000$ ft³/d is distributed as a specified flux boundary condition in the nodes located about 40 ft from the center of the quadrant (i.e., location of well in analytical solution); remaining boundary nodes are specified as no-flow boundaries.

benchmark: Hantush-Jacob (1955) analytical solution for drawdown with given transmissivity, storativity, leakance and pumping rate:

$$s = \frac{Q}{4\pi T} W(u, v)$$

where

$$u = \frac{r^2 S}{tT}$$

and

$$v = \frac{r}{2} \sqrt{\frac{k'}{b'T}}$$

where, Q = flow rate [L^3/T], T = transmissivity [L^2/T] = $k_h b$, S = storativity [fraction], k_h = horizontal hydraulic conductivity of aquifer [L/T], b = thickness of aquifer [L], k' = vertical hydraulic conductivity of confining layer [L/T], b' = thickness of confining layer [L], r = distance from well [L], t = time since start of pumping [T], s = drawdown with respect to prepumping horizontal piezometric surface [L], and W = the Leakance Function [dimensionless]. Calculation are made for observation wells at $r = 200, 650$ and 1000 ft from the well, respectively.

grid: Single layer 1/2-quadrant (45°) grid (see Figure B5-2) with three levels at 0, 20, and 30 ft, respectively; in plan view, the grid consists of 25 nodes defining 30 elements.

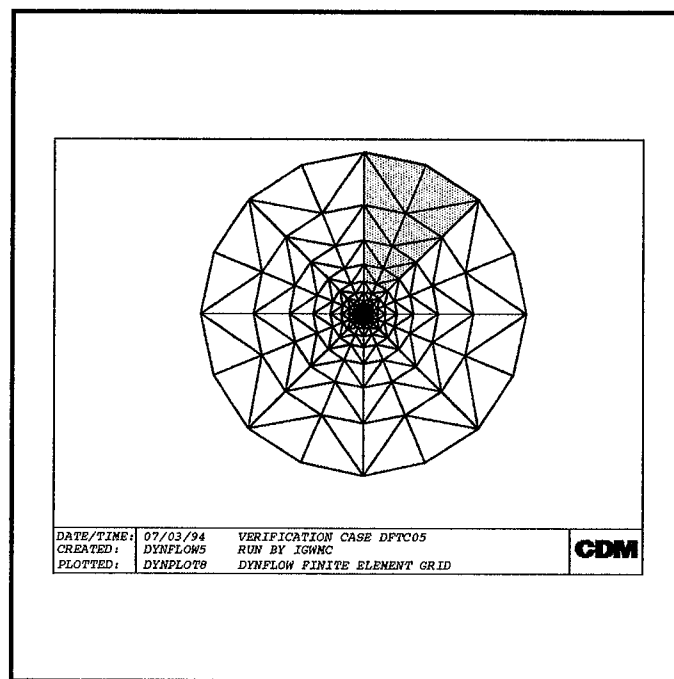


Figure B5-2.

initial conditions: $h=100$ ft

time-stepping: Preset time steps of 0.5 days, which is greater than the final time of computation; actual time step set such that each comparison time is a single time step from previous comparison time.

system parameters: Hydraulic conductivity $k_x = k_y = k_h = 500$ ft/d, $k_z = 1$ ft/d; specific storativity = 0.00005 ft⁻¹; aquifer specific yield = 0.10; $k' = 1.0$ ft/d; specific storativity of aquitard was set at 0.00000001 ft⁻¹ (a value of zero as suggested in the documentation caused a runtime error).

control parameters: Tolerance = .0001 ft; alpha = 1.25 (relaxation factor); acce = 1.5; max. # outer iterations = 30; max. # inner iterations = 90.

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Graphic plot of heads (see Fig. B5-3); tabular listing of heads (see Table B5-1, and B5-2; no statistical measures calculated).

Table B5-1.

time (days)	# iterations	drawdown (ft)					
		r = 200 ft (node 4)		r = 650 ft (node 9)		r = 1000 ft (node 12)	
		Hantush	DF ¹⁾	Hantush	DF ¹⁾	Hantush	DF ¹⁾
$t_1 = 0.005$	8	1.246	1.18	0.041	.05	0.001	0
$t_2 = 0.0125$	9	1.700	1.96 - 1.97	0.174	.14	0.026	.02
$t_3 = 0.02375$	9	1.840	1.95 - 1.96	0.254	.25	0.058	.05
$t_4 = 0.040625$	9	1.872	2.05 - 2.06	0.280	.26	0.073	.07
$t_5 = 0.0659375$	9	1.876	1.99	0.285	.26	0.076	.06
$t_6 = 0.1$	9	1.876	2.04 - 2.05	0.285	.26	0.076	.07

1) For each time, the range of values at different levels is given, if applicable

Table B5-2. Global fluxes in ft³/d

time step >	t ₁	t ₂	t ₃	t ₄	t ₅	t ₆
influx including storage	20006.770	21094.440	21078.300	21146.030	20998.310	20857.220
outflux	20004.710	21093.170	21083.340	21146.380	20997.910	20859.160
total mass balance error	.01%	.01%	.02%	.001%	.002%	.01%

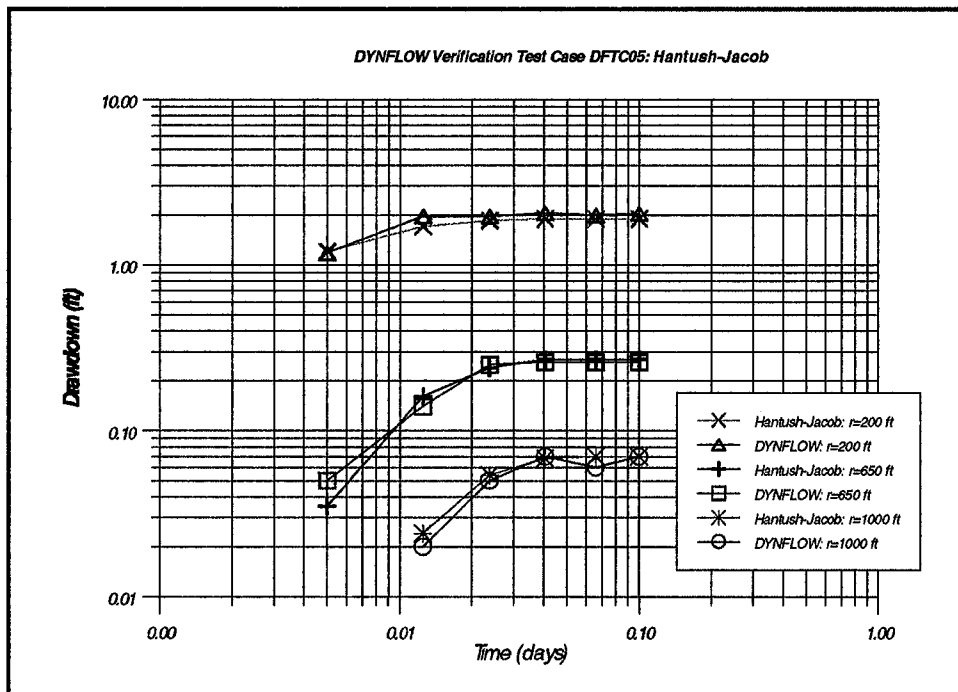


Figure B5-3.

performance notes: Although the resulting heads differ somewhat from the benchmark, they are reasonably small. The mass balance error is very small for each of the time steps, although the flux varies slightly (note that pumping is constant).

Command File DFTC05.CFI

OUTPUT DFTC05.OUT

!
 TITLE
 VERIFICATION CASE NO. 5 -- HANTUSH; RUN BY PVDH, BOULDER, CO.
 HANTU TRANSIENT CONFINED 2D RADIAL HORIZ FLOW WITH LEAKAGE.
 REF.: LOHMAN, "GROUND-WATER HYDRAULICS," USGS PROF PAPER 708, PAGES 8.
 TEXT

LET:

Kh = HORIZ HYDRAULIC CONDUCTIVITY
 = 500.
 Kv = VERT HYDRAULIC CONDUCTIVITY
 = K' = 1.0
 b = AQUIFER THICKNESS
 = 20.
 b' = AQUITARD THICKNESS
 = 10.
 Ss = SPECIFIC STORATIVITY
 = 0.00005
 Q = WELL PUMPING RATE
 = 160000.

SOLUTION:

OBTAIN IT BY USING THE PLATES GIVEN IN LOHMAN'S PAPER,
 WHICH FOR THREE POINTS IN AQUIFER IS AS FOLLOWS (APPX.):

NODE POINT	4	9	12
DISTANCE	200.	650.	1000.
NU PARAMETER	0.316	1.03	1.60

TIME	DRAWDOWN		
.005	1.2	0.035	N/A
.0125	1.7	0.16	0.024
.02375	1.85	0.24	0.054
.040625	1.9	0.27	0.068
.0659375	1.9	0.27	0.07
.1	1.9	0.27	0.07

ENDT
 GRID READ DFTC05.GRF FORM
 LEVEL 3.
 FREE
 ELEV 0. LEVELSING 1
 ELEV 20. LEVELSING 2
 ELEV 30. LEVELSING 3
 PROP
 1,500.,500.,500.,0.00005,0.0,0.,0.,0.
 ELEM 301. LAYERSING 1
 PROP
 2,1.,1.,1.,0.00000001,0.,0.,0.,0.
 ELEM 302. LAYERSING 2
 INIT 100.
 FIX LEVELRANGE 1 2NODE RANGE 23 25
 FIX LEVELSING 3
 FLUX -10000. LEVELRANGE 1 2NODE SING 1
 DT 0.005
 TOL .0001
 ACCE 1.5
 PRAL
 ALPHA 1.25
 ITIN 90.
 ITER 30.
 HEAD LEVELSING 1 NODE SING 4 SAVE DFTC4.T4 NOHD

Test Summary: DYNFLOW - Version 5.18 (September 1996)

HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .005						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .0125						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .02375						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .040625						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .0659375						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
GOTIL .1						
HEAD	LEVELSING	1	NODE SING	4	SAVE DFTC4.T4	NOHD
HEAD	LEVELSING	1	NODE SING	9	SAVE DFTC4.T9	NOHD
HEAD	LEVELSING	1	NODE SING	12	SAVE DFTC4.T12	NOHD
XCFL						

Grid File DFTC05.GRF

25	30						
1	-100.	0.					
2	-92.388	38.2683					
3	-92.388	-38.2683			15	-1600.	0.
4	-200.	0.			16	-1478.207	-612.2935
5	-323.3578	133.9392			17	-2771.639	1148.0503
6	-350.	0.			18	-3000.	0.
7	-323.3578	-133.9392			19	-2771.639	-1148.050
8	-600.5217	248.7442			20	-5543.227	2296.1006
9	-650.	0.			21	-6000.	0.
10	-600.5217	-248.7442			22	-5543.227	-2296.101
11	-923.88	382.683			23	-11086.55	4592.2012
12	-1000.	0.			24	-12000.	0.
13	-923.88	-382.683			25	-11086.55	-4592.201
14	-1478.207	612.2935					
1	1	2	4	17	14	15	12
2	1	4	3	18	12	15	16
3	2	5	4	19	14	17	15
4	3	4	7	20	15	19	16
5	5	6	4	21	17	18	15
6	4	6	7	22	15	18	19
7	5	8	6	23	17	20	18
8	6	10	7	24	18	22	19
9	8	9	6	25	20	21	18
10	6	9	10	26	18	21	22
11	8	11	9	27	20	23	21
12	9	13	10	28	21	25	22
13	11	12	9	29	23	24	21
14	9	12	13	30	21	24	25
15	11	14	12				
16	12	16	13				

Test 6: ONE-DIMENSIONAL CONSOLIDATION

test #: DFTC06A, DFTC06B

command file: DFTC06A.CFI, DFTC06B.CFI

grid file: DFTC06.GRF

output file: DFTC06A.OUT, DFTC06B.OUT

date of test execution: November 1996

title: One-dimensional flow through consolidating aquitard.

description: Transient vertical flow (free drainage) through a consolidating aquitard due to a falling water level on top; aquitard is isotropic and homogeneous (see figure B6-1).

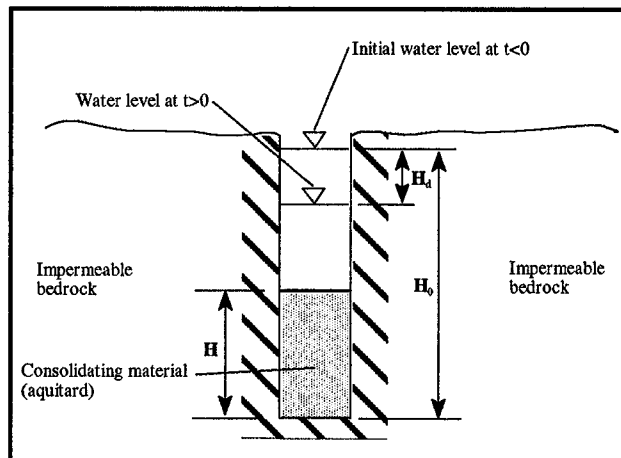


Figure B6-1.

tested functions: Transient vertical flow (free drainage); stress-dependent parameters (permeability, porosity), fixed head at lower boundary.

model domain: Column with length $H = 100$ ft (aquitard thickness) and unit width (in both X- and Y-direction) = 1 ft.

boundary conditions: Lateral no-flow condition; fixed head at bottom = 150 ft (representing head drop $H_d = 50$ ft).

benchmark: Graphic representation of Lambe and Whitman, tabularized by DYNFLOW authors.

grid: Two-element (4 nodes) in plan view; eight layers (see Figure B3-2) with levels at 0, 12.5, 25.0, 37.5, 50.0, 62.5, 75.0, 87.5, and 100.0 ft, respectively.

initial conditions: $H_0 = 200$ ft

Test Summary: DYNFLOW - Version 5.18 (September 1996)

- time-stepping: Preset time steps of 200 days (DFTC06A) and 100 days (DFTC06B) over a period of 9,000 days.
- system parameters: Hydraulic conductivity $k_x = k_y = k_h = 1.0$ ft/d, $k_z = 1.0$ ft/d; specific storativity = 1.0 ft⁻¹; aquifer specific yield = 1.0.
- control parameters: Tolerance = .001 ft; alpha = 1.8 (relaxation factor); acce = (not given); max. # outer iterations = 30; max. # inner iterations = 50.
- solver: ICCG
- test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.
- type of comparison: Graphic plot of heads (see Fig. B6-3); tabular listing of heads (see Table B6-1, and B6-2; no statistical measures calculated.

Table B6-1.

time (days)	# iterations	head (ft)					
		level 5 (1/4 point of leaky layer)			level 9 (midpoint of leaky layer)		
		benchmark	DF		benchmark	DF	
			$\Delta t = 200$ days	$\Delta t = 100$ days		$\Delta t = 200$ days	$\Delta t = 100$ days
1000	20	187.0	186.68	186.58	197.2	197.37	197.41
2000	20	177.5	177.56	177.53	188.5	188.52	188.49
3000	20	170.3	171.41	171.39	180.0	180.23	180.21
4000	20	166.7	166.71	166.71	173.7	173.63	173.61
5000	20	162.7	163.05	163.04	168.5	168.45	168.44
6000	20	160.0	160.19	160.18	164.3	164.41	164.40
7000	20	157.7	157.96	157.95	161.2	161.25	161.24
8000	20	155.8	156.21	156.21	158.8	158.78	158.78
9000	20	154.6	154.85	154.84	156.8	156.86	156.85

Test Summary: DYNFLOW - Version 5.18 (September 1996)

performance notes: Although the resulting heads differ somewhat from the benchmark presented by the DYNFLOW authors, they are reasonably small (max 1.1 ft or 0.6%). Differences between the results for time steps of 100 days and 200 days are very small. Mass balance error has not been calculated as all fluxes are zero.

Command File DFTC06A.CFI (Note that the only difference in DFTC06B.CFI is DT=100 of DT=200).

OUTPUT DFTC06A.OUT

!

TITLE

VERIFICATION CASE NO. 6A -- CONSOLIDATION; RUN BY PVDH, BOULDER, CO.
TRANSIENT CONFINED 1D VERTICAL FLOW WITH PRESSURE-DEPENDENT PERMEABILITY.
REF.: LAMBE AND WHITMAN, "SOIL MECHANICS," PAGES 406-410.
TEXT

TERZAGHI'S FAMOUS SOLUTION.

LET:

k = HYDRAULIC CONDUCTIVITY
= 1.0
Ss = SPECIFIC STORATIVITY
= 1.0
Sy = SPECIFIC YIELD
= 1.0
H0 = INITIAL HEAD
= 200.0
Hd = HEAD DROP
= 50.0

SOLUTION:

SEE LAMBE AND WHITMAN, WHERE THE 1D CONSOLIDATION CURVES ARE PRESENTED IN A GRAPH. THE APPX. SOLUTION FOR TWO POINTS, ONE AT THE MIDDLE OF THE LEAKY LAYER AND THE OTHER AT THE ONE-QUARTER (OR THREE-QUARTER) POINT IS AS FOLLOWS:

TIME	MID-POINT	1/4 - POINT
-----	-----	-----
1000.	197.2	187.0
2000.	188.5	177.5
3000.	180.0	170.3
4000.	173.7	166.7
5000.	168.5	162.8
6000.	164.3	160.0
7000.	161.2	157.7
8000.	158.8	155.8
9000.	156.8	154.6

ENDT

GRID READ DFTC06.GRF FORM

LEVEL 9.

FREE

PROP

1,1.,1.,1.0,1.0,1.0,0.,0.,0.

ELEM 301.

ELEV 0.	LEVEL	1	NODE ALL
ELEV 12.5	LEVEL	2	NODE ALL
ELEV 25.	LEVEL	3	NODE ALL
ELEV 37.5	LEVEL	4	NODE ALL
ELEV 50.	LEVEL	5	NODE ALL
ELEV 62.5	LEVEL	6	NODE ALL
ELEV 75.	LEVEL	7	NODE ALL
ELEV 87.5	LEVEL	8	NODE ALL
ELEV 100.	LEVEL	9	NODE ALL
INIT 200.			
INIT 150.	LEVELSING	1	

Test Summary: DYNFLOW - Version 5.18 (September 1996)

```

FIX          LEVELSING      1
DT          200.
TOL         0.001
NOPR
ALPHA      1.8
ITER       30.
ITIN       50.
C00000000111111111122222222233333333334444444445555555556666666667777777778
C234567890123456789012345678901234567890123456789012345678901234567890
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
GOTIL      1000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
GOTIL      2000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
GOTIL      3000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
GOTIL      4000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
GOTIL      5000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
GOTIL      6000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
GOTIL      7000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
GOTIL      8000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
GOTIL      9000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
GOTIL     10000.
HEAD        LEVELSING      5      NODE SING      1      SAVE DFTC6.T5 NOHD
HEAD        LEVELSING      9      NODE SING      1      SAVE DFTC6.T9 NOHD
PRIN
XCFI

```

Grid File DFTC06.GRF

```

      4      2
      1      0.0      0.0
      2      1.0      0.0
      3      1.0      1.0
      4      0.0      1.0

      1      1      2      3
      2      1      3      4

```

Test 7: SEEPAGE FACE

tests #: DFTC07

command files: DFTC07.CFI

grid files: DFTC07.GRF

output files: DFTC07.OUT

date of test execution: October 1996

title: SEEPAGE: Horizontal one-dimensional unconfined flow with seepage face

description: Steady-state horizontal one-dimensional flow between a reservoir and a seepage boundary in an isotropic, homogeneous, unconfined aquifer in the presence of a seepage face (see figure B7-1).

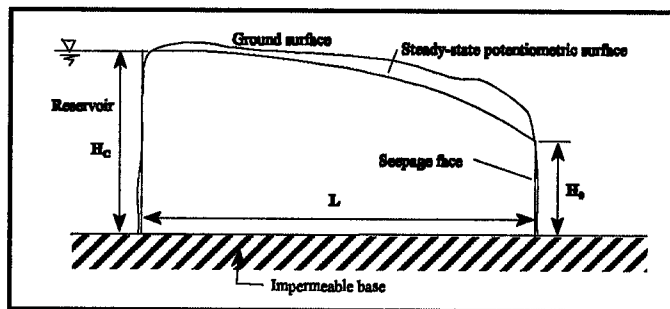


Figure B7-1.

tested functions: Steady-state horizontal unconfined flow; node point head and flux computation; rising water algorithm; fixed head boundary condition; seepage face.

model domain: Strip between two parallel drains with length and width of $L = 100$ ft, $w = 10$ ft, respectively.

boundary conditions: Fixed head at left boundary $h_c = 100$ ft; although height of tail-water equals zero, fixed head at right boundary ($L=100$ ft) is set at 0.01 ft (at bottom layer only); other right side boundary nodes have rising water condition.

benchmark: Analytical solution by Polibarinova-Kochina (1962) for height of seepage face and discharge at seepage face.

grid: Three-dimensional grid with nine levels and eight layers; in plan view, the grid consists of 33 nodes in three rows, defining 40 elements (see figure B7-2).

initial conditions: n.a. (steady state)

time-stepping: n.a. (steady state)

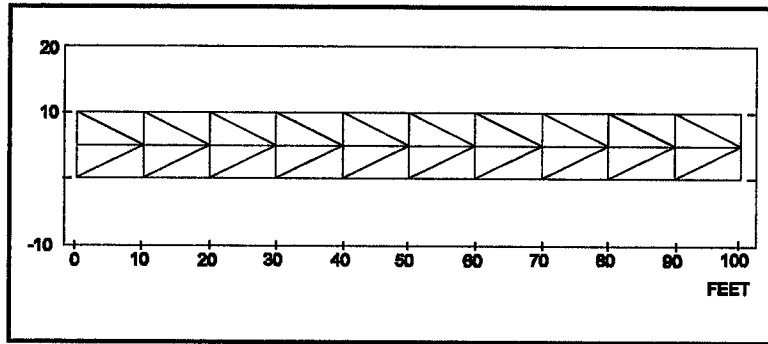


Figure B7-2.

system parameters: Hydraulic conductivity $K_x = K_y = K_z = 1.0$ ft/d.

control parameters: Tolerance = .1 ft; alpha = 1.5 (relaxation factor); acc not defined; max. # outer iterations = 30; max. # inner iterations = 50.

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Tabular listing of heads (see Table B7-1).

Table B7-1

	Seepage Face Height (ft)	Discharge (cu. ft/day)
Benchmark	37.5	500.0
DYNFLOW	37.65 (node 1) 37.50 (node 2) 36.31 (node 3) 37.15 (average)	inflow: 500.037 outflow: 500.046 mass balance error: 0.002%

performance note: Convergence reached in 13 outer iterations.

Command File DFTC07.CFI

OUTPUT DFTC07.OUT

!

TITLE

VERIFICATION CASE NO. 7 -- SEEPAGE FACE

SEEPAGE STEADY-STATE PHREATIC 1D FLOW WITH SEEPAGE FACE.

REF.: POLUBARINOVA-KOCHINA.

TEXT

WHERE:

H = HEAD IN RESERVOIR.

h0 = SEEPAGE FACE LENGTH.

h = TAILWATER ELEVATION.

Q = FLOW THROUGH DAM.

k = HYDRAULIC CONDUCTIVITY.

l = DISTANCE FROM RESERVOIR TO TAILWATER, THROUGH DAM.

LET:

H = 100.0

h = 0.0

l = 100.0

k = 1.0

SOLUTION:

SEE FIGURES 220 - 225 IN REFERENCE, ON PAGES 292 - 294.

THEY GIVE THE FOLLOWING SOLUTION --

H₀ = 37.5

Q = 500.0

ENDT

GRID READ DFTC07.GRF FORM

!

LEVEL 9.

FREE

PROP

1,1.,1.,1.,0.,0.,0.0,0.,0.

ELEM 301.

ELEV 0. LEVELSING 1

ELEV 12.5 LEVELSING 2

ELEV 25.0 LEVELSING 3

ELEV 37.5 LEVELSING 4

ELEV 50.0 LEVELSING 5

ELEV 62.5 LEVELSING 6

ELEV 75.0 LEVELSING 7

ELEV 87.5 LEVELSING 8

ELEV 110.0 LEVELSING 9

INIT 100.00

INIT 0.01 LEVELSING 1 NODE RANGE 1 3

RISI LEVELALL NODE RANGE 1 3

FIX LEVELALL NODE RANGE 31 33

FIX LEVELSING 1 NODE RANGE 1 3

DT 0.

TOL 0.1

PRAL

ALPHA 1.5

ITIN 50.

ITER 30.

GOTIL 0.

C00000000111111111222222222233333333334444444445555555555666666666677777777778

C2345678901234567890123456789012345678901234567890123456789012345678901234567890

HEAD LEVELSING 9 NODE RANGE 1 3SAVE DFTC09.CHK NOHD

XCFI

Grid File DFTC07.GRF

33	40						
1	0	10.0		18	50.0	0	
2	0	5.0		19	60.0	10.0	
3	0	0		20	60.0	5.0	
4	10.0	10.0		21	60.0	0	
5	10.0	5.0		22	70.0	10.0	
6	10.0	0		23	70.0	5.0	
7	20.0	10.0		24	70.0	0	
8	20.0	5.0		25	80.0	10.0	
9	20.0	0		26	80.0	5.0	
10	30.0	10.0		27	80.0	0	
11	30.0	5.0		28	90.0	10.0	
12	30.0	0		29	90.0	5.0	
13	40.0	10.0		30	90.0	0	
14	40.0	5.0		31	100.0	10.0	
15	40.0	0		32	100.0	5.0	
16	50.0	10.0		33	100.0	0	
17	50.0	5.0					
1	5	4	1	21	20	19	16
2	2	5	1	22	17	20	16
3	3	5	2	23	18	20	17
4	3	6	5	24	18	21	20
5	8	7	4	25	23	22	19
6	5	8	4	26	20	23	19
7	6	8	5	27	21	23	20
8	6	9	8	28	21	24	23
9	11	10	7	29	26	25	22
10	8	11	7	30	23	26	22
11	9	11	8	31	24	26	23
12	9	12	11	32	24	27	26
13	14	13	10	33	29	28	25
14	11	14	10	34	26	29	25
15	12	14	11	35	27	29	26
16	12	15	14	36	27	30	29
17	17	16	13	37	32	31	28
18	14	17	13	38	29	32	28
19	15	17	14	39	30	32	29
20	15	18	17	40	30	33	32

Test 8: MOUND

test #: DFTC08

input file: DFTC08.CFI

grid file: DFTC08.GRF

output file: DFTC08.OUT

date of test execution: November 1996

title: MOUND: horizontal one-dimensional unconfined (non-linear) flow (Dupuit) due to uniform recharge.

description: Steady-state horizontal flow between two fully penetrating canals with identical surface potentials in an isotropic, homogeneous, unconfined aquifer with a horizontal impermeable base subject to uniform recharge on top (see figure B8-1).

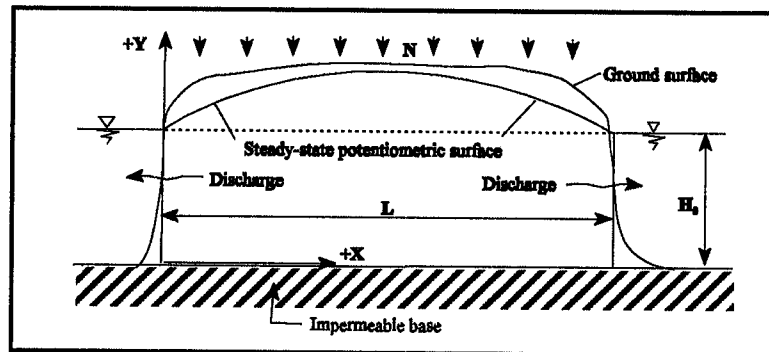


Fig. B8-1.

tested functions: Steady-state horizontal unconfined flow; node point head and flux computations; fixed head boundary condition; areal recharge.

model domain: Strip between two parallel drains with a length, width and thickness of $L=100$ ft, and $w=10$ ft, respectively.

boundary conditions: Constant head at left and right boundary $h_0=50$ ft; no-flow boundaries in y-direction, and at bottom; uniform recharge (flux) at top boundary.

benchmark: Analytical solution for a nonlinear potentiometric surface between two identical fixed head boundaries in a unconfined aquifer under steady-state conditions and uniform recharge:

$$h = \sqrt{h_0^2 + \frac{N}{k} (L - x)x}$$

and the discharge is calculated from:

$$Q = \frac{N}{2} L W$$

where N = recharge rate [L/T], Q = flow rate [L³/T], k = horizontal hydraulic conductivity [L/T], H_0 = fixed head at boundaries [L], L = length of aquifer [L], and W = width of aquifer [L].

grid: Grid with five layer and six levels (0, 20, 40, 60, 80, and 200 ft); in plan view, the grid consists of 33 nodes in three rows, defining 40 elements (see figure B8-2); the horizontal definition of the grid is identical to the one used in the LINEAR test problem; heads are fixed below the top level to simulate unconfined conditions.

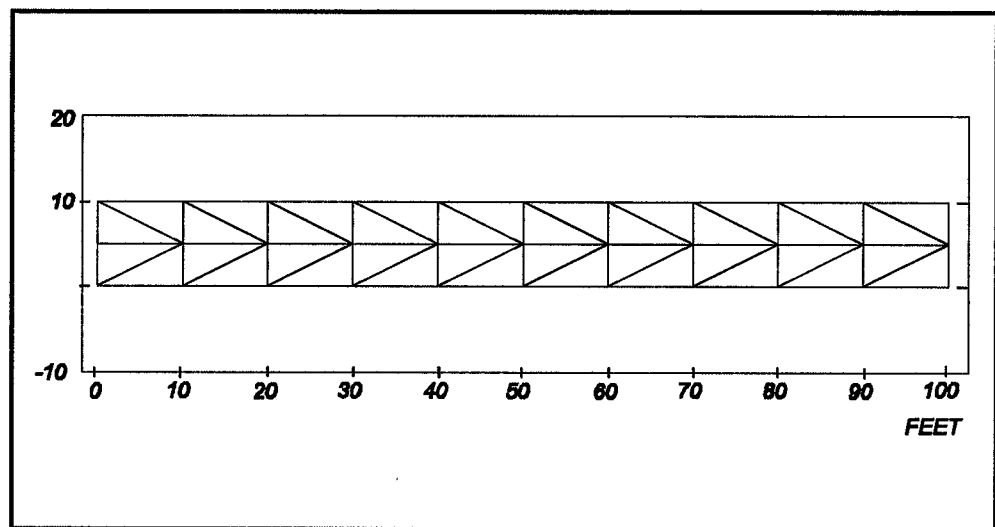


Figure B8-2.

initial conditions: n.a. (steady state)

time-stepping: n.a. (steady state)

system parameters: Hydraulic conductivity $K_x = K_y = 1.0$ ft/d; $K_z = 100.0$ ft/d; Recharge rate $N = 1.25$ ft/d.

control parameters: Tolerance = .001 ft; alpha = 1.5 (relaxation factor); acc not used; max. # outer iterations = 30; max. # inner iterations = 90

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Graphic plot of heads (see Fig. B8-3); tabular listing of heads (see Table B8-1); no statistical measures calculated.

Table B8-1

distance from downstream end (ft)	piezometric head (ft)						
	analytical solution	DYNFLOW DFTC08					
		level 1	level 2	level 3	level 4	level 5	level 6
0	50.	50. (FH)	50. (FH)	50. (FH)	50. (FH)	50. (FH)	50. (FH)
10	60.21	60.01	60.07	60.25	60.56	60.65	60.65
20	67.08	66.90	66.94	67.08	67.31	67.42	67.42
30	71.92	71.41	71.45	71.57	71.76	71.91	71.91
40	74.16	73.99	74.03	74.13	74.31	74.47	74.47
50	75.00	74.83	74.87	74.97	75.14	75.31	75.31
60	74.16	73.99	74.03	74.14	74.31	74.48	74.48
70	71.92	71.42	71.46	71.57	71.77	71.92	71.92
80	67.08	66.90	66.95	67.08	67.32	67.43	67.43
90	60.21	60.01	60.07	60.26	60.60	60.70	60.69
100	50.	50. (FH)	50. (FH)	50. (FH)	50. (FH)	50. (FH)	50. (FH)
maximum difference with benchmark per level [ft]		.51	.47	.35	.39	.49	.48
FH - Fixed Head Node (The numbers listed for each level are the average of three nodal values)							

Table B8-2

max # of outer iterations	6
influx	1250.149 ft ³ /d
outflux	1250.149 ft ³ /d
total mass balance error	.0%
max. head difference	.7%

performance notes: Resulting heads differ locally from the benchmark presented by the DYNFLOW authors (up to 0.7%; see Fig. B8-3). Computed flow rates for inflow and outflow (1250.149 ft³/d) differ about 0.012% from the analytical results (2*625.0 ft³/d).

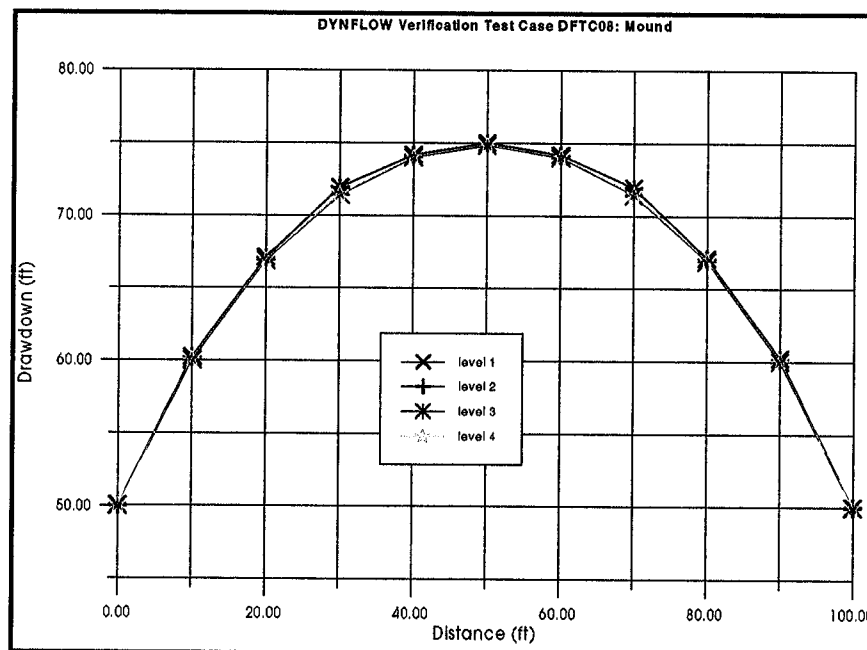


Figure B8-3.

Command File DFTC08.CFI

```

OUTPUT DFTC08.OUT
!
TITL
VERIFICATION CASE NO. 8 -- MOUND; RUN BY PVDH, BOULDER, CO.
DUPUIT STEADY-STATE UNCONFINED 1D HORIZ FLOW WITH UNIFORM RECHARGE
REF.: WILSON IN BSCES/ASCE "GROUNDWATER HYDROLOGY," 1981.
TEXT

  H^2 = H0^2 + ( N / k ) ( L - x ) x
  Q   = ( N / 2 ) L W
    
```

Test Summary: DYNFLOW - Version 5.18 (September 1996)

WHERE:

H = HEAD AT ANY POINT x.
x = DISTANCE FROM ONE END OF THE AQUIFER TO THE OTHER.
0 = POINT MARKER FOR "LEFT" OR "RIGHT" END OF AQUIFER.
H0 = HEAD AT POINT 0.
N = RECHARGE.
k = HYDRAULIC CONDUCTIVITY.
L = LENGTH OF AQUIFER.
^2 = SQUARE.
W = WIDTH.

LET:

H0 = 50.0
N = 1.25
L = 100.0
k = 1.0
W = 10.0

SOLUTION:

$$H^2 = 2500 + 125 x - 1.25 x^2$$

x	H
10.	60.21
20.	67.08
30.	71.59
40.	74.16
50.	75.00
60.	74.16
70.	71.59
80.	67.08
90.	60.21

$$Q = 625.0$$

ENDT

GRID READ DFTC08.GRF FORM

!

LEVEL 6.

FREE

PROP

1,1.,1.,100.,0.,0.,1.25,0.,0.

PROP

2,1.,1.,100.,0.,0.,1.25,0.,0.

ELEM 301. LAYERRANGE 1 3

ELEM 302. LAYERRANGE 4 6

ELEV 0. LEVELSING 1

ELEV 20. LEVELSING 2

ELEV 40. LEVELSING 3

ELEV 60. LEVELSING 4

ELEV 80. LEVELSING 5

ELEV 200. LEVELSING 6

INIT 50.

FIX LEVELALL NODE RANGE 1 3

FIX LEVELALL NODE RANGE 31 33

DT 0.

TOL .001

PRAL

!DEBUG 1.

ALPHA 1.5

ITIN 90.

ITER 30.

GOTIL 0.

Test Summary: DYNFLOW - Version 5.18 (September 1996)

C0000000011111111222222222223333333333344444444445555555555666666666677777777778
 C2345678901234567890123456789012345678901234567890123456789012345678901234567890
 HEAD LEVELSING 1 NODE LIST SAVE DFTC10.CHK
 2, 5, 8, 11, 14, 17, 20, 23, 26, 29, 32,
 END
 XCFI

Grid File DFTC08.GRF

25	30						
1	-100.	0.			14	-1478.207	612.2935
2	-92.388	38.2683			15	-1600.	0.
3	-92.388	-38.2683			16	-1478.207	-612.2935
4	-200.	0.			17	-2771.639	1148.0503
5	-323.3578	133.9392			18	-3000.	0.
6	-350.	0.			19	-2771.639	-1148.050
7	-323.3578	-133.9392			20	-5543.227	2296.1006
8	-600.5217	248.7442			21	-6000.	0.
9	-650.	0.			22	-5543.227	-2296.101
10	-600.5217	-248.7442			23	-11086.55	4592.2012
11	-923.88	382.683			24	-12000.	0.
12	-1000.	0.			25	-11086.55	-4592.201
13	-923.88	-382.683					
1	1	2	4	16	12	16	13
2	1	4	3	17	14	15	12
3	2	5	4	18	12	15	16
4	3	4	7	19	14	17	15
5	5	6	4	20	15	19	16
6	4	6	7	21	17	18	15
7	5	8	6	22	15	18	19
8	6	10	7	23	17	20	18
9	8	9	6	24	18	22	19
10	6	9	10	25	20	21	18
11	8	11	9	26	18	21	22
12	9	13	10	27	20	23	21
13	11	12	9	28	21	25	22
14	9	12	13	29	23	24	21
15	11	14	12	30	21	24	25

Test 9: CONFINED-UNCONFINED STORAGE CONVERSION

test #: DFTC09

command file: DFTC09N.CFI

grid file: DFTC09N.GRF

output file: DFTC09N.OUT

date of test execution: January 5, 1998

title: Confined-unconfined storage conversion

description: Transient radial-symmetric flow towards a fully penetrating pumping well with a constant discharge rate in an isotropic, homogeneous, porous medium of infinite extent and constant thickness; some time after pumping starts, the piezometric level in the initially fully confined aquifer decreases below the top of the aquifer and, locally, conditions become unconfined (see figure B9-1).

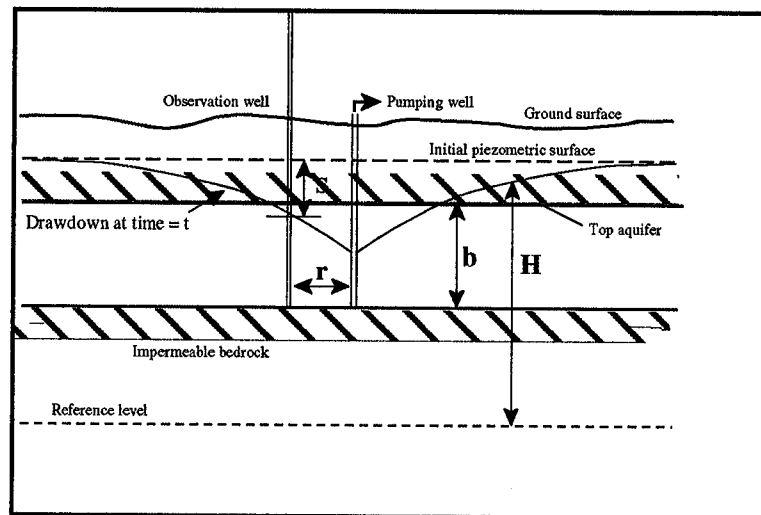


Figure B9-1.

tested functions: Transient horizontal flow under variably confined conditions; discharging well; unconfined/confined storage factor conversion.

model domain: *Analytical solution:* infinite horizontal domain and thickness of 800 ft; *numerical solution:* 1/2-quadrant (45°) with radius of 24,000 ft. and thickness of 800 ft.

benchmark: Analytical solution by Moench and Prickett (1972) for drawdown in a single, nonleaky, homogeneous, isotropic aquifer of infinite extent with water level conversion from confined to phreatic conditions; the well fully penetrates the aquifer and is infinitesimal in diameter:

$$h_1 = b - \frac{Q}{4\pi T} [W(u_1) - W(v)]$$

and

$$h_2 = H - \frac{(H-b) W(u_2)}{W(v\alpha_1/\alpha_2)}$$

where

$$W(x) = \int_z^\infty \frac{e^{-z}}{z} dz \quad [\textit{dimensionless}]$$

$$u_1 = \frac{r^2 S_1}{4tT} \quad [\textit{dimensionless}]$$

$$u_2 = \frac{r^2 S_2}{4tT} \quad [\textit{dimensionless}]$$

$$v = \frac{R^2 S_1}{4tT} \quad [\textit{dimensionless}]$$

and Q = pumping rate [L^3/T], T = aquifer transmissivity [L^2/T] = $k_h b$, k_h = horizontal hydraulic conductivity of aquifer [L/T], b = thickness of aquifer [L], S_1 = storativity under water table conditions [fraction], S_2 = storativity under confined conditions [fraction], α_1 = aquifer diffusivity (= T/S_1) [L^2/T], α_2 = aquifer diffusivity (= T/S_2) [L^2/T], r = distance from well [L], R = radial distance to point of conversion [L], t = time since start of pumping [T], h_1 = elevation of water table (unconfined conditions) from bottom aquifer [L], h_2 = elevation of piezometric surface (confined conditions) from bottom aquifer [L].

Calculations are made for observation wells at $r = 100, 350, 650,$ and 1000 ft from the well.

grid: Single layer 1/2-quadrant (45°) grid of 24,000 ft radius and two levels (see Figure B9-2) with actual location of well as origin of radial-symmetrical grid; in plan view, the grid consists of 30 nodes defining 36 elements; levels are located at elevation 0.0 and 800.0 ft, respectively.

boundary conditions: Fixed head at outer radial boundary ($R=24,000$ ft) is $h_R = 802$ ft; total well discharge rate $Q_{tot} = 33648$ ft³/d; $Q_m = 1/8$ of Q_{tot} is distributed as a specified flux boundary condition in the 3 nodes located about 100 ft from the center of the quadrant at both level 1 and 2, *i.e.*, $1/6$ of Q_m per node or $1/48$ of Q_{tot} (= 701 ft³/d); remaining boundary nodes are specified as no-flow boundaries.

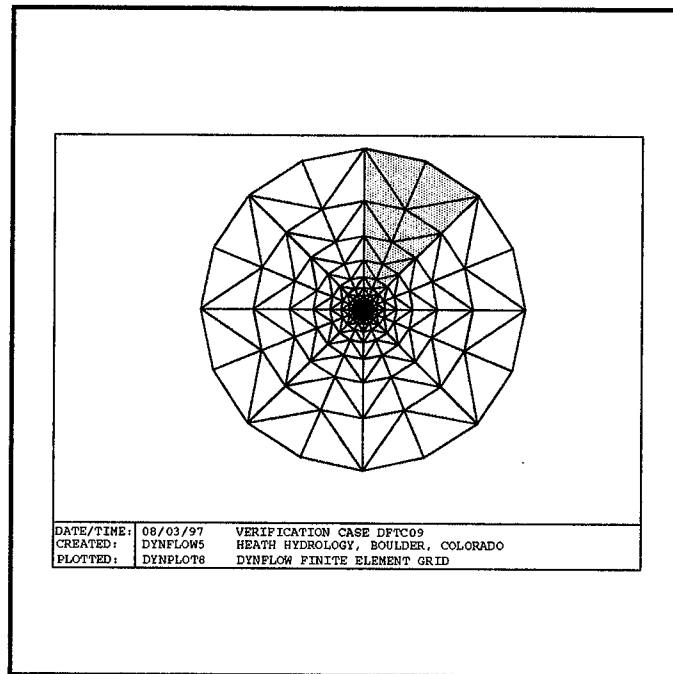


Figure B9-2.

initial conditions: $H = 802.0$ ft, with H being 2.0 ft above top of top level.

time-stepping: Calculations made for 5 time steps : 0.1, 1.0, 10.0, 100.0, and 1000.0 days.

system parameters: Hydraulic conductivity $k_x = k_y = K_h = 3.34$ ft/d; $k_z = K_v = 33.4$ ft/d; specific storativity $S_s = 1.25 \times 10^{-6}$ ft⁻¹ (storage coefficient $S = S_s \cdot b = 0.001$; dimensionless); storativity or specific yield for unconfined conditions = 0.10 (dimensionless).

control parameters: Tolerance = .001 ft; acce = 1.4; max. # outer iterations = 30.

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Graphic plot of heads (see Fig. B9-3); tabular listing of heads (see Table B9-1, and B9-2; no statistical measures calculated).

Table B9-1.

time (days)	drawdown (ft)							
	r =200 ft		r=350 ft		r=650 ft		r=1000 ft	
	M-P ¹⁾	DF ²⁾ average nodes 5,6,7	M-P ¹⁾	DF ²⁾ average nodes 8,9,10	M-P ¹⁾	DF ²⁾ average nodes 11,12,13	M-P ¹⁾	DF ²⁾ average nodes 14,15,16
t ₁ = 0.1	1.28	1.63	0.79	1.08	0.33	0.48	0.11	0.17
t ₂ = 1.0	2.42	2.55	1.82	1.92	1.26	1.34	0.88	0.93
t ₃ = 10.0	4.42	4.33	3.38	3.31	2.39	2.35	1.91	1.91
t ₄ = 100.0	6.70	6.50	5.59	5.42	4.37	4.26	3.56	3.48
t ₅ = 1000.0	9.00	8.82	7.88	7.74	6.64	5.54	5.79	5.71

1) M-P: Moench-Prickett solution (MathCAD)
 2) DYNFLOW : for each time, the range of values (for different levels) is given

Table B9-2. Global fluxes in ft³/d

time step	t ₁	t ₂	t ₃	t ₄	t ₅
# iterations	2	2	2	2	3
influx including storage	4290.2	4369.8	4457.9	4512.8	4597.0
outflux	4290.2	4379.1	4456.1	4512.4	4593.4
total mass balance error	0.01%	0.21%	0.04%	0.01%	0.08%

performance notes: In comparing DYNFLOW with the benchmark, a trend can be observed: for short times the drawdown computed is greater than that of the benchmark, while for long times, the reverse is the case. The mass balance is generally good, except for time equals 1 day. The discrepancies between the code and the benchmark may result in part from inaccuracies in the execution of the benchmark. In programming the benchmark in Mathcad (MathSoft, Inc.), it was tested against the data given in the paper by Moench and Prickett (1972). The actual benchmark used in this test case uses modified data.

reference: Moench, A.F., and T.A. Prickett. (1972). Radial Flow in an Infinite Aquifer Undergoing Conversion from Artesian to Water Table Conditions. Water Resources Research, Vol. 8(2), pp. 494-499.

Command File DFTC09N.CFI

OUTPUT DFTC09N.OUT

! RHF Revisions:

! Extended grid (24,000 foot radius) (DFTC09N.GRF)
! 1-layer, 2-level model
! pumping evenly distributed between levels and among 3 nodes
! at center of grid
! Do not fix far boundary nodes

TITLE

VERIFICATION CASE NO. 9 -- MIXED CONFINED/UNCONFINED; RUN BY PVDH, BOULDER, CO.
TRANSIENT CONFINED AND UNCONFINED CONDITIONS (MIXED) EXTENDED GRID
REF.: MOENCH AND PRICKETT (1972) WRR 8(2):494-499.
TEXT

LET:

Kh = HORIZ HYDRAULIC CONDUCTIVITY
= 3.34
Kv = VERT HYDRAULIC CONDUCTIVITY
= 10*kh
b = AQUIFER THICKNESS
= 800.
Ss = CONFINED STORATIVITY
= 0.00000125
Sy = UNCONFINED STORATIVITY
= 0.10
Q = WELL PUMPING RATE
= 33648.
ALSO, LET THE INITIAL HEAD (H) = 802, SO THAT
H - b = 2.0

ENDT

GRID DFTC09N.GRF FORM

LEVEL 2.
FREE NODE ALL
ELEV 0. LEVELSING 1
ELEV 800. LEVELSING 2
PROP
1,3.34,3.34,33.4,0.00000125,0.10,0.
MATNUM 301. ELEM ALL LAYER ALL
INIT 802. NODE ALL LAYER ALL
FLUX -701 LEVEL 1 2 NODE 2 3 4
ITER 30.
ACCE 1.4
TOL 0.001
PRAL
NOPR
HEAD LEVEL 1 2 NODE 14 15 16 SAVE DFTC9.T0 NOHD
DT .01
GOTIL .1
HEAD LEVEL 1 2 NODE 14 15 16 SAVE DFTC9.TP1 NOHD
PRIN
DT 0.1
GOTIL 1.0
HEAD LEVEL 1 2 NODE 14 15 16 SAVE DFTC9.T1 NOHD
PRIN
DT 1.0
GOTIL 10.0
HEAD LEVEL 1 2 NODE 14 15 16 SAVE DFTC9.T10 NOHD
PRIN
DT 10.0
GOTIL 100.0
HEAD LEVEL 1 2 NODE 14 15 16 SAVE DFTC9.T1C NOHD

Test Summary: DYNFLOW - Version 5.18 (September 1996; Built June 4, 1997)

```

PRIN
DT      100.0
GOTIL 1000.0
HEAD      LEVEL      1  2      NODE      14 15 16      SAVE DFTC9.T1K NOHD
PRIN
END
    
```

Grid File DFTC09N.GRF

30	36						
2	100.0	.000			17	1600.	.000
3	92.39	38.27			18	1478.	612.3
4	70.71	70.71			19	1131.	1131.
5	200.0	.000			20	3000.	.000
6	184.8	76.54			21	2772.	1148.
7	141.4	141.4			22	2121.	2121.
8	350.0	.000			23	6000.	.000
9	323.4	133.9			24	5543.	2296.
10	247.5	247.5			25	4243.	4243.
11	650.0	.000			26	12000.	.000
12	600.5	248.7			27	11087.	4592.
13	459.6	459.6			28	8485.	8485.
14	1000.	.000			29	24000.	.000
15	923.9	382.7			30	22173.	9184.
16	707.1	707.1			31	16971.	16971.

3	2	5	3	21	15	19	16
4	5	6	3	22	15	18	19
5	3	7	4	23	17	20	18
6	3	6	7	24	20	21	18
7	5	8	6	25	18	22	19
8	8	9	6	26	18	21	22
9	6	10	7	27	20	23	21
10	6	9	10	28	23	24	21
11	8	11	9	29	21	25	22
12	11	12	9	30	21	24	25
13	9	13	10	31	23	26	24
14	9	12	13	32	26	27	24
15	11	14	12	33	24	28	25
16	14	15	12	34	24	27	28
17	12	16	13	35	26	29	27
18	12	15	16	36	29	30	27
19	14	17	15	37	27	31	28
20	17	18	15	38	27	30	31

Test 10: RISING/FALLING WATER TABLE (WETTING FRONT)

PART A: RISING WATER TABLE (RESATURATION)

tests #: DFTC10A

command files: DFTC10AN.CFI

grid file: DFTC10N.GRF

output files: DFTC10AN.OUT

date of test execution: January 6, 1998

title: Rising water-table (wetting front or resaturation)

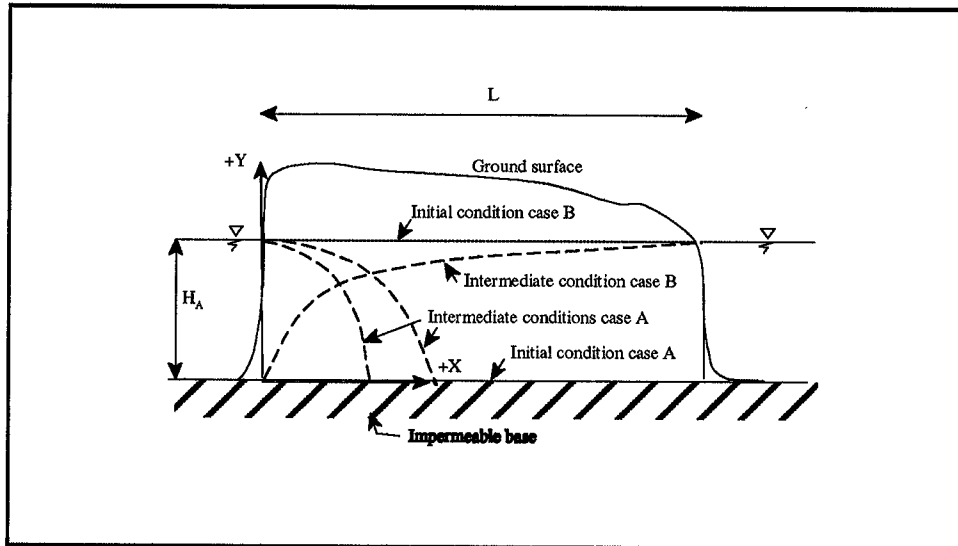


Figure B10-1.

description: Transient flow in a multi-layer, rectangular isotropic, homogeneous, unconfined aquifer; flow starts after sudden rise of fixed head boundary condition at $T = 0$ at one side of the semi-infinite model domain (from $H = 0$ to $H = H_A$).

In the numerical model, the fixed head at the opposite side remains constant during the simulation ($H = 0$) assuming that this boundary is far enough away from the opposite boundary to have a negligible influence on the distribution of heads within the aquifer for the time period selected; no-flow conditions exist on the other two parallel sides (see figure B10-1).

- tested functions: Unconfined potentiometric surface calculation (correctness/accuracy); (re-)wetting of dry elements by rising water table (correctness/stability).
- model domain: Strip between two parallel drains with a length $L=100$ ft and a width $W=10$ ft.
- boundary conditions: Constant head at left boundary ($x = 0$ ft) is $h_0 = 10$ ft; constant head at right boundary ($x = 100$ ft) is $h_L = 0$ ft; no-flow boundaries in y - and z -direction.
- benchmark: Transient solution as given by Polubarinova-Kochina (1962) and programmed in Mathcad (MathSoft, Inc.) using the following dimensionless variables:
- $$= [(x^2 * S_y) / (2 * K * H_0 * t)]^{1/2}; \quad = / \sqrt{2}; \quad u = H(x,t)/H_0$$
- where S_y = specific yield, K = horizontal hydraulic conductivity, H_0 = boundary condition at surface water boundary, x = distance from surface water boundary, and t = time since change occurred in boundary condition.
- The Mathcad implementation has been based on the equations presented in pp. 508-510 of Polubarinova-Kochina (1962); the values used for comparison are those listed in Table 17 and Fig. 378 on p. 509 (Mathcad worksheet PK-508.mcd). For comparison with the numerical model, a new data set has been created Mathcad worksheet PK-508a.mcd to keep the progressing toe within a small section of the model domain away from the constant head boundary.
- Note: The values for the parameters and time stepping are chosen such that the effect of the sudden change in head at one boundary has not reached the opposite boundary by the end of the simulation.
- grid: Two-dimensional horizontal grid with six levels and five layers; each layer is 2 ft thick, except for the top layer which is set at 3 ft; in plan view, the grid consists of 63 nodes in three rows, defining 80 elements per layer (see figure B10-2).
- initial conditions: $h = 0$ ft.
- time-stepping: At the start $DT= 0.02$ days until time = 1.0 days, then $DT=0.05$ until time = 5 days; finally, $DT =0.1$ until time = 10 days (the small initial time steps are chosen to keep the mass balance error small and allow the program to converge within the iteration bounds).
- system parameters: Hydraulic conductivity $K_x = K_y = 1.0$ ft/d; $K_z = 100$ ft/d; specific yield $S_y = 0.1$.
- control parameters: Tolerance = .001 ft; alpha = 1.5 (relaxation factor); max. # outer iterations = 30; max. # inner iterations = 50.
- solver: ICCG
- test performed by: Problem set up for numerical code, code run and benchmark comparison performed by test report author.

type of comparison: Tabular listing of heads (see Table B10-1 and B10-2); iteration progress, steady-state discharge calculation; no statistical measures calculated.

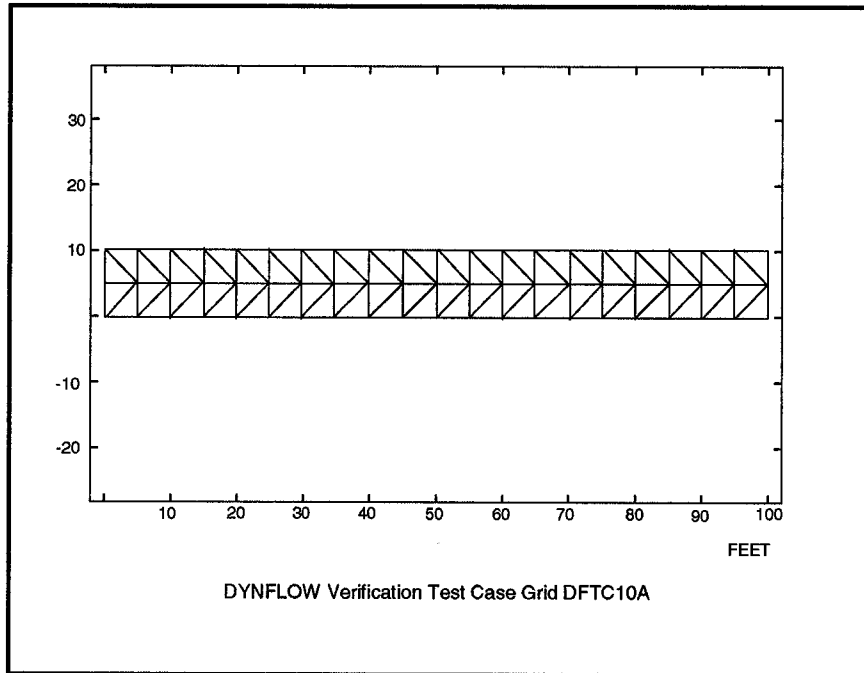


Figure B10-2.

Table B10-1: Results Case A - Head comparison for rising water table at various times

<i>X</i>	Time = 2 days			Time = 5 days			Time = 10 days		
	<i>PK</i> ¹⁾	<i>DF</i> ²⁾	ΔH ³⁾	<i>PK</i> ¹⁾	<i>DF</i> ²⁾	ΔH ³⁾	<i>PK</i> ¹⁾	<i>DF</i> ²⁾	ΔH ³⁾
10	6.35	6.38 [6.38]	0.03	7.81	7.82 [7.82]	0.01	8.50	8.50 [8.50]	0.
20	1.58	1.48 [1.66]	-0.10	5.20	5.21 [5.21]	0.01	6.78	6.79 [6.79]	0.01
30	<0	0.	0.	2.13	2.13 [2.15]	0.	4.85	4.86 [4.86]	0.01
40	<0	0.	0.	<0	.01 [.02]	0.01	2.69	2.69 [2.69]	0.
50	<0	0.	0.	<0	0.	0.	0.28	0.32 [0.41]	0.04
60	<0	0.	0.	<0	0.	0.	<0	0.	0.
70	<0	0.	0.	<0	0.	0.	<0	0.	0.
80	<0	0.	0.	<0	0.	0.	<0	0.	0.
90	<0	0.	0.	<0	0.	0.	<0	0.	0.

- 1) Analytical solution according to Polibarinova-Kochina
- 2) DYNFLOW results are given as the average head in all relevant nodes (i.e., having the same distance from origin) for all levels followed in brackets by the average head in the nodes on the center grid line for all levels.
- 3) Difference between numerical model and benchmark; positive number indicates overprediction by numerical model.

Table B10-2: Code Performance.

test case DF10A			
time	2 days	5 days	10 days
# of iterations	3	2	2
influx [ft ³ /d]	40.593	19.723	14.054
outflux [ft ³ /d]	-40.588	-19.719	-14.039
total mass balance error	0.01%	0.02%	0.10%

performance notes: To obtain an accurate mass balance, initial time steps had to be reduced to very small values and the grid refined (original grid for this case was identical to that for case 10B). The head comparison is excellent, including in the area near the infiltration front.

reference: Polubarinova-Kochina, P.Ya. 1962. Theory of Groundwater Movement. Princeton University Press, Princeton, New Jersey, 613p.

Command File DFTC10AN.CFI for test DFTC10A

OUTPUT DFTC10AN.OUT

```
! RHF Revisions:
!   New grid with 5-foot node spacing in x-direction
!   Lumped storage option selected, i.e. storage flux not distributed
!   to adjacent nodes
```

TITLE

```
VERIFICATION CASE NO. 10A -- RISING WATER TABLE (WETTING FRONT/REWETTING)
TRANSIENT UNCONFINED FLOW WITH SUDDEN CHANGE IN FIXED HEAD BOUNDARY CONDITION
RUN BY PVDH, BOULDER, CO. -- INCREASED GRID RESOLUTION
```

GRID READ DFTC10N.GRF FORM

```
LEVEL 6.
FREE
PROP
1,1.,1.,100.,0.,0.1,0.
ELEM 301.
ELEV 0.      LEVELSING      1
ELEV 2.      LEVELSING      2
ELEV 4.      LEVELSING      3
ELEV 6.      LEVELSING      4
ELEV 8.      LEVELSING      5
ELEV 11.     LEVELSING      6
INIT 0.0     NODE ALL LEVEL ALL
INIT 10.     LEVELALL        NODE RANGE 1 3
FIX         LEVELALL        NODE RANGE 1 3
TOL .001
ITER 30.
ITIN 50.
PRAL
NOPR
DT 0.02
```

```
! LUMPED STORAGE OPTION
STORE 1
```

```
GOTIL 1.0
DT 0.05
GOTIL 2.0
HEAD          LEVELSING      6      NODE ALL          SAVE NEW10.T2 NOHD
PRIN
GOTIL 5.0
HEAD          LEVELSING      6      NODE ALL          SAVE NEW10.T5 NOHD
PRIN
DT=0.1
GOTIL 10.0
HEAD          LEVELSING      6      NODE ALL          SAVE NEW10.T10 NOHD
PRIN
END
```

Grid File DFTC10N.GRF

63	80			
1	.000	.000	53	85.00 5.000
2	.000	5.000	54	85.00 10.00
3	.000	10.00	55	90.00 .000
4	5.000	.000	56	90.00 5.000
5	5.000	5.000	57	90.00 10.00
6	5.000	10.00	58	95.00 .000
7	10.00	.000	59	95.00 5.000
8	10.00	5.000	60	95.00 10.00
9	10.00	10.00	61	100.0 .000
10	15.00	.000	62	100.0 5.000
11	15.00	5.000	63	100.0 10.00
12	15.00	10.00		
13	20.00	.000		
14	20.00	5.000		
15	20.00	10.00		
16	25.00	.000		
17	25.00	5.000		
18	25.00	10.00		
19	30.00	.000		
20	30.00	5.000		
21	30.00	10.00		
22	35.00	.000		
23	35.00	5.000		
24	35.00	10.00		
25	40.00	.000		
26	40.00	5.000		
27	40.00	10.00		
28	45.00	.000		
29	45.00	5.000		
30	45.00	10.00		
31	50.00	.000		
32	50.00	5.000		
33	50.00	10.00		
34	55.00	.000		
35	55.00	5.000		
36	55.00	10.00		
37	60.00	.000		
38	60.00	5.000		
39	60.00	10.00		
40	65.00	.000		
41	65.00	5.000		
42	65.00	10.00		
43	70.00	.000		
44	70.00	5.000		
45	70.00	10.00		
46	75.00	.000		
47	75.00	5.000		
48	75.00	10.00		
49	80.00	.000		
50	80.00	5.000		
51	80.00	10.00		
52	85.00	.000		

1	1	5	2	41	31	35	32
2	2	5	3	42	32	35	33
3	1	4	5	43	31	34	35
4	3	5	6	44	33	35	36
5	4	8	5	45	34	38	35
6	5	8	6	46	35	38	36
7	4	7	8	47	34	37	38
8	6	8	9	48	36	38	39
9	7	11	8	49	37	41	38
10	8	11	9	50	38	41	39
11	7	10	11	51	37	40	41
12	9	11	12	52	39	41	42
13	10	14	11	53	40	44	41
14	11	14	12	54	41	44	42
15	10	13	14	55	40	43	44
16	12	14	15	56	42	44	45
17	13	17	14	57	43	47	44
18	14	17	15	58	44	47	45
19	13	16	17	59	43	46	47
20	15	17	18	60	45	47	48
21	16	20	17	61	46	50	47
22	17	20	18	62	47	50	48
23	16	19	20	63	46	49	50
24	18	20	21	64	48	50	51
25	19	23	20	65	49	53	50
26	20	23	21	66	50	53	51
27	19	22	23	67	49	52	53
28	21	23	24	68	51	53	54
29	22	26	23	69	52	56	53
30	23	26	24	70	53	56	54
31	22	25	26	71	52	55	56
32	24	26	27	72	54	56	57
33	25	29	26	73	55	59	56
34	26	29	27	74	56	59	57
35	25	28	29	75	55	58	59
36	27	29	30	76	57	59	60
37	28	32	29	77	58	62	59
38	29	32	30	78	59	62	60
39	28	31	32	79	58	61	62
40	30	32	33	80	60	62	63

PART B: FALLING WATER TABLE (DESATURATION)

tests #: DFTC10B

command files: DFTC10B.CFI

grid files: DFTC10.GRF

output files: DFTC10B.OUT

date of test execution: October 8, 1997

title: Falling water-table (desaturation/dewatering)

description: Transient flow in a multi-layer, rectangular isotropic, homogeneous, unconfined aquifer; flow after sudden fall of fixed head (from $H = H_A$ to $H = 0$) at $T=0$ at one side of the semi-infinite model domain.

In the numerical model, the fixed head at the opposite side remains constant during the simulation ($H = H_A$) assuming that this boundary is far enough away from other the opposite boundary to have a negligible influence on the distribution of heads within the aquifer for the time period selected; no-flow conditions exist on the other two parallel sides (see figure B10-1).

tested functions: Unconfined potentiometric surface calculation (correctness/accuracy); drainage of wet elements (dewatering) by falling water table (correctness/stability).

model domain: Strip between two parallel drains with a length $L=100$ ft and a width $W=10$ ft.

boundary conditions: Constant head at left boundary ($x = 0$ ft) is $h_0 = 0$ ft; constant head at right boundary ($x = 100$ ft) is $h_L = 10$ ft; no-flow boundaries in y- and z-direction.

benchmark: Transient solution as given by Polubarinova-Kochina (1962) and programmed in Mathcad (MathSoft, Inc.) using the following dimensionless variables:

$$= [(x^2 * S_y) / (2 * K * H_0 * t)]^{1/2}; \quad = / \sqrt{2}; \quad u = H(x,t)/H_0$$

where S_y = specific yield, K = horizontal hydraulic conductivity, H_0 = boundary condition at surface water boundary, x = distance from surface water boundary, and t = time since change occurred in boundary condition.

The mathcad implementation has been based on the equations presented in pp. 506-507 of Polubarinova-Kochina (1962); the values used for comparison are those listed in Table 17 and Fig. 377 on p. 508 (MathCAD worksheet PK-506.mcd). For comparison with the

numerical model, a new data set has been created Mathcad worksheet PK-506a.mcd to keep the progressing toe within a small section of the model domain away from the constant head boundary.

Note: The values for the parameters and time stepping are chosen such that the effect of the sudden change in head at one boundary has not reached the opposite boundary by the end of the simulation.

grid: Two-dimensional horizontal grid with six levels and five layers; each layer is 2 ft thick, except for the top layer which is set at 3 ft; in plan view, the grid consists of 33 nodes in three rows, defining 40 elements per layer (see figure B10-3).

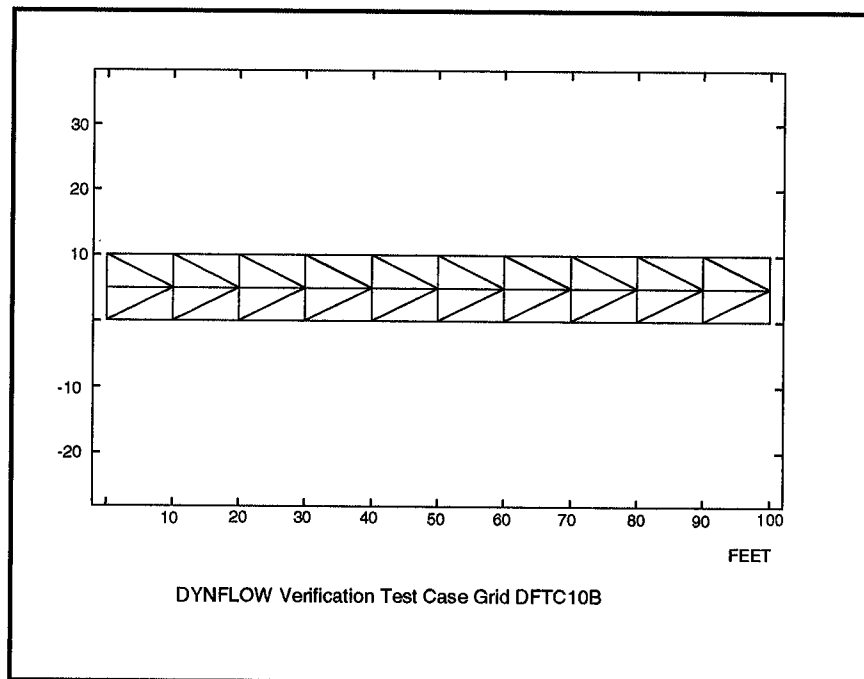


Figure B10-3.

initial conditions: $h = 10$ ft.

time-stepping: At the start $DT = 0.02$ days until time = 1.0 days, then $DT = 0.05$ until time = 5 days; finally, $DT = 0.1$ until time = 10 days (small initial time steps are chosen to keep the mass balance error small and allow the program to converge within the iteration bounds).

system parameters: Hydraulic conductivity $K_x = K_y = 1.0$ ft/d; $K_z = 100$ ft/d; specific yield $S_y = 0.1$.

control parameters: tolerance = .001 ft; $\alpha = 1.5$ (relaxation factor); max. # outer iterations = 30; max. # inner iterations = 50.

solver: ICCG

test performed by: Problem set up for numerical code, code run and benchmark comparison performed by test report author.

type of comparison: Tabular listing of heads (see Table B10-3 and B10-4); iteration progress, steady-state discharge calculation; no statistical measures calculated.

Table B10-3: Results Case B - Head comparison for falling water table

<i>X</i>	Time = 10 days		
	<i>PK</i> ¹⁾	<i>DF</i> ²⁾	ΔH ³⁾
10	4.52	4.45-4.53 [4.47]	-0.05
20	6.22	6.18-1.19 [6.18]	-0.04
30	7.37	7.33-7.34 [7.33]	-0.04
40	8.19	8.16 [8.16]	-0.03
50	8.78	8.75 [8.75]	-0.03
60	9.20	9.17 [9.17]	-0.03
70	9.49	9.46 [9.46]	-0.03
80	9.68	9.64 [9.64]	-0.04
90	9.80	9.75 [9.75]	-0.05
100	9.87	9.78 [9.78]	-0.09
<p>1) Analytical solution according to Polibarinova-Kochina</p> <p>2) DYNFLOW results are given as a range for all relevant nodes (i.e., same distance from origin) followed by the average value between brackets.</p> <p>3) Difference between numerical model and benchmark; negative number indicates underprediction by numerical model.</p>			

performance notes: This problem is easier to solve for the code than the rising water table case; head deviations and mass balance errors are very small, especially for larger time.

reference: Polubarinova-Kochina, P.Ya. 1962. Theory of Groundwater Movement. Princeton University Press, Princeton, New Jersey, 613p.

Table B10-4: Code Performance.

test case	DFTC10B	
	2 days	10 days
# of iterations outer/(inner)	6(2)	6(2)
influx [ft ³ /d]	74.306	63.475
outflux [ft ³ /d]	-74.307	-63.478
total mass balance error	0.001%	0.005%

Command File DFTC10B.CFI for test DFTC10B

OUTPUT DFTC10B.OUT

```

TITLE
VERIFICATION CASE NO. 10B -- FALLING WATER TABLE (DEWATERING)
TRANSIENT UNCONFINED FLOW WITH SUDDEN CHANGE IN FIXED HEAD BOUNDARY CONDITION
RUN BY PVDH, BOULDER, CO.
GRID READ DFTC10.GRF FORM
LEVEL 6.
FREE
PROP
1,1.,1.,100.,0.,0.1,0.
ELEM 301.
ELEV 0. LEVELSING 1
ELEV 2. LEVELSING 2
ELEV 4. LEVELSING 3
ELEV 6. LEVELSING 4
ELEV 8. LEVELSING 5
ELEV 11. LEVELSING 6
INIT 10. NODE ALL LAYER ALL
INIT 0. LEVELALL NODE RANGE 1 3
FIX LEVELALL NODE RANGE 1 3
TOL .001
ALPHA 1.5
ITER 30.
ITIN 50.
PRAL
NOPR
DT 0.02
GOTIL 1.0
DT 0.05
GOTIL 2.0
HEAD LEVELSING 6 NODE ALL SAVE DFTC10B.T6 NOHD
    
```

```

PRIN
GOTIL 5.0
PRIN
HEAD          LEVELSING      6      NODE ALL          SAVE DFTC10B.T6 NOHD
DT=0.1
GOTIL 10.0
HEAD          LEVELSING      6      NODE ALL          SAVE DFTC10B.T6 NOHD
PRIN
END
    
```

Grid File DFTC10.GRF for test case DFTC10B

33	40	5					
1	0	10.0		18	50.0	0	
2	0	5.0		19	60.0	10.0	
3	0	0		20	60.0	5.0	
4	10.0	10.0		21	60.0	0	
5	10.0	5.0		22	70.0	10.0	
6	10.0	0		23	70.0	5.0	
7	20.0	10.0		24	70.0	0	
8	20.0	5.0		25	80.0	10.0	
9	20.0	0		26	80.0	5.0	
10	30.0	10.0		27	80.0	0	
11	30.0	5.0		28	90.0	10.0	
12	30.0	0		29	90.0	5.0	
13	40.0	10.0		30	90.0	0	
14	40.0	5.0		31	100.0	10.0	
15	40.0	0		32	100.0	5.0	
16	50.0	10.0		33	100.0	0	
17	50.0	5.0					
1	5	4	1	21	20	19	16
2	2	5	1	22	17	20	16
3	3	5	2	23	18	20	17
4	3	6	5	24	18	21	20
5	8	7	4	25	23	22	19
6	5	8	4	26	20	23	19
7	6	8	5	27	21	23	20
8	6	9	8	28	21	24	23
9	11	10	7	29	26	25	22
10	8	11	7	30	23	26	22
11	9	11	8	31	24	26	23
12	9	12	11	32	24	27	26
13	14	13	10	33	29	28	25
14	11	14	10	34	26	29	25
15	12	14	11	35	27	29	26
16	12	15	14	36	27	30	29
17	17	16	13	37	32	31	28
18	14	17	13	38	29	32	28
19	15	17	14	39	30	32	29
20	15	18	17	40	30	33	32

Test 11: SPHERICAL FLOW

test #: DFTC11 (SPHERE)

command file: DFTC11.CFI (SPHER.CFI)

grid file: DFTC11.GRF (SPHER.GRF)

output file: DFTC11.OUT (SPHER.OUT)

date of test execution: July 8, 1999

title: Steady-state spherical flow away from a point source in a three-dimensional infinite space.

description: Steady-state flow away from an injection well with an infinite small screen located in the center of a three-dimensional infinite domain; the recharge rate is held constant; the aquifer is isotropic, homogeneous and confined.

tested functions: Fully three-dimensional steady-state flow under confined conditions (i.e., symmetrical flow); fixed head condition at the lateral, top and bottom boundaries; recharging well with constant flux; mass balance calculations.

model domain: *Analytical solution*: the model domain is the three-dimensional infinite space centered around the point well; *numerical model*: the model domain is the cubical positive X-, Y- and Z-space, bounded at a distance of 104 ft in the three principal directions; the well is located in the origin of the coordinate system.

boundary conditions: Fixed head at the outer boundary as read from a file, reflecting effects of non-infinite boundary; fixed injection rate at well node is 400 ft³/d.

benchmark: Analytical solution of the form (Strack 1990, p. 215):

$$H = Q_0 / 4\pi KR_d + H_i / K \quad \text{with} \quad R_d = \sqrt{X^2 + Y^2 + Z^2}$$

H_i = initial condition; K = hydraulic conductivity; Q_0 = pumping rate

grid: Twelve-layer grid with thirteen levels spaced in the same fashion as the x- and y-grid (see figure B11-1 and table B11-1); in plan view, the grid consists of 115 nodes defining 186 elements (see file DFTC11.GRF or SPHER.GRF).

initial conditions: Read from file DFTC11.HDS.

time-stepping: n.a. (steady-state).

system parameters: $k_x = k_y = k_z = 1.0$ ft/d; [specific storativity = 0.0 ft⁻¹; aquifer specific yield = 0.00; not used in steady-state].

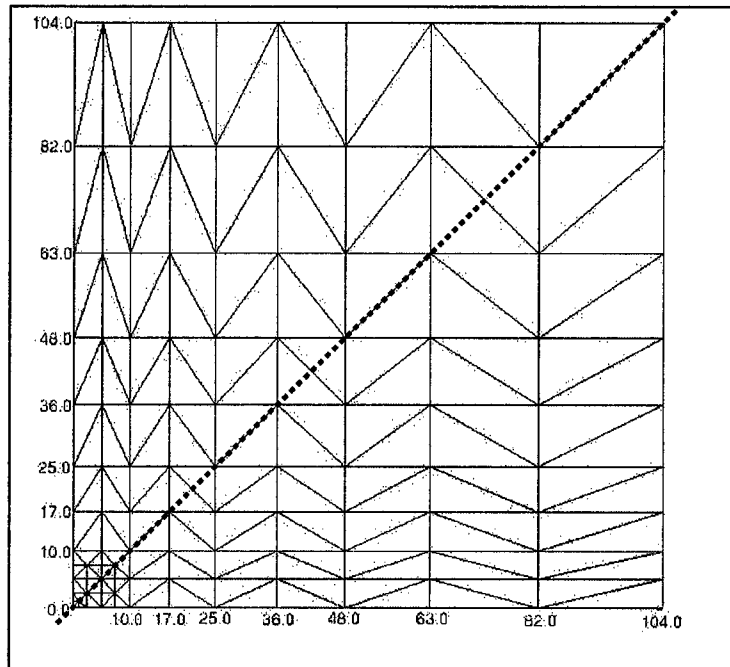


Figure B11-1. Horizontal grid definition (X - Y plane).

Table B11-1. Vertical grid definition (Z direction).

Level	Z-coordinate	Level	Z-coordinate
1	0.0	2	1.0
3	2.5	4	5.0
5	7.5	6	10.0
7	17.0	8	25.0
9	36.0	10	48.0
11	63.0	12	82.0
13	104.0		

verification points: The main verification points are in the vertical plane located at 45° from the X- and Y-axis (see hatched line in figure B11-1). Additional check points are located in the X-Z and Y-Z planes (see table B11-2). The latter check points are compared with equivalent points in the X-Y plane for asymmetrical tendencies.

Table B11-2. Coordinates of comparison nodes.

Node number	X-coordinate	Y-coordinate	Comments
2	0.0	5.0	symmetry check at level 4
4	0.0	17.0	symmetry check at level 7
8	0.0	63.0	symmetry check at level 11
11	5.0	0.0	symmetry check at level 4
12	5.0	5.0	bench mark check at all levels
31	17.0	0.0	symmetry check at level 7
34	17.0	17.0	bench mark check at all levels
71	63.0	0.0	symmetry check at level 11
78	63.0	63.0	bench mark check at all levels
101	2.5	2.5	bench mark check at all levels
102	2.5	0.0	symmetry check at level 3
103	0.0	2.5	symmetry check at level 3
113	1.0	1.0	bench mark check at all levels
114	0.0	1.0	symmetry check at level 2
115	1.0	0.0	symmetry check at level 2

control parameters: Tolerance = 0.001 ft; alpha = 1.5 (relaxation factor); max. # inner iterations = 50.

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code test run and benchmark comparison made by test report author.

type of comparison: Tabular listing of heads (see Table B11-3) and graphical comparison of relative differentials (i.e., difference between DYNFLOW result and bench mark divided by bench mark value; see Figure B11-2); no statistical measures calculated.

Table B11-3. Comparison of results from DYNFLOW with bench mark.

Level	Node	Distance	Bench Mark	DYNFLOW	Difference (DYNFLOW-bench mark)
1	113	1.41	22.51	20.02	-2.48
2	113	1.73	18.38	15.45	-2.92
3	113	2.87	11.08	9.98	-1.10
4	113	5.20	6.13	5.87	-0.25
5	113	7.63	4.17	4.02	-0.15
6	113	10.10	3.15	2.99	-0.17
7	113	17.06	1.87	1.85	-0.01
8	113	25.04	1.27	1.28	0.01
9	113	36.03	0.88	0.90	0.01
10	113	48.02	0.66	0.67	0.01
11	113	63.02	0.51	0.51	0.01
12	113	82.01	0.39	0.39	0.01
13	113	104.01	0.31	0.31	0.00
1	101	3.54	9.00	8.58	-0.43
2	101	3.67	8.66	8.19	-0.48
3	101	4.33	7.35	6.91	-0.44
4	101	6.12	5.20	5.00	-0.19
5	101	8.29	3.84	3.72	-0.12
6	101	10.61	3.00	2.86	-0.14
7	101	17.36	1.83	1.83	-0.01
8	101	25.25	1.26	1.27	0.01
9	101	36.17	0.88	0.89	0.01
10	101	48.13	0.66	0.67	0.01
11	101	63.10	0.50	0.51	0.01
12	101	82.08	0.39	0.39	0.01
13	101	104.06	0.31	0.31	0.00

Level	Node	Distance	Bench Mark	DYNFLOW	Difference (DYNFLOW-bench mark)
1	12	7.07	4.50	4.42	-0.09
2	12	7.14	4.46	4.35	-0.11
3	12	7.50	4.24	4.11	-0.14
4	12	8.66	3.68	3.51	-0.16
5	12	10.31	3.09	2.96	-0.13
6	12	12.25	2.60	2.51	-0.09
7	12	18.41	1.73	1.71	-0.02
8	12	25.98	1.23	1.23	0.00
9	12	36.69	0.87	0.88	0.01
10	12	48.52	0.66	0.67	0.01
11	12	63.40	0.50	0.51	0.01
12	12	82.30	0.39	0.39	0.01
13	12	104.24	0.31	0.30	0.00
1	34	24.04	1.32	1.30	-0.02
2	34	24.06	1.32	1.30	-0.02
3	34	24.17	1.32	1.30	-0.02
4	34	24.56	1.30	1.28	-0.02
5	34	25.18	1.26	1.25	-0.01
6	34	26.04	1.22	1.21	-0.01
7	34	29.44	1.08	1.07	-0.01
8	34	34.68	0.92	0.91	-0.01
9	34	43.29	0.74	0.73	0.00
10	34	53.68	0.59	0.60	0.00
11	34	67.43	0.47	0.48	0.01
12	34	85.45	0.37	0.38	0.00
13	34	106.74	0.30	0.30	0.00
1	78	89.10	0.36	0.36	0.00
2	78	89.10	0.36	0.36	0.00
3	78	89.13	0.36	0.36	0.00
4	78	89.24	0.36	0.36	0.00
5	78	89.41	0.36	0.36	0.00
6	78	89.65	0.36	0.36	0.00
7	78	90.70	0.35	0.35	0.00
8	78	92.54	0.34	0.35	0.00
9	78	96.09	0.33	0.34	0.00
10	78	101.20	0.31	0.32	0.01
11	78	109.12	0.29	0.30	0.01
12	78	121.09	0.26	0.27	0.01
13	78	136.95	0.23	0.23	0.00

iterations: 37

Water balance error: 0.00%

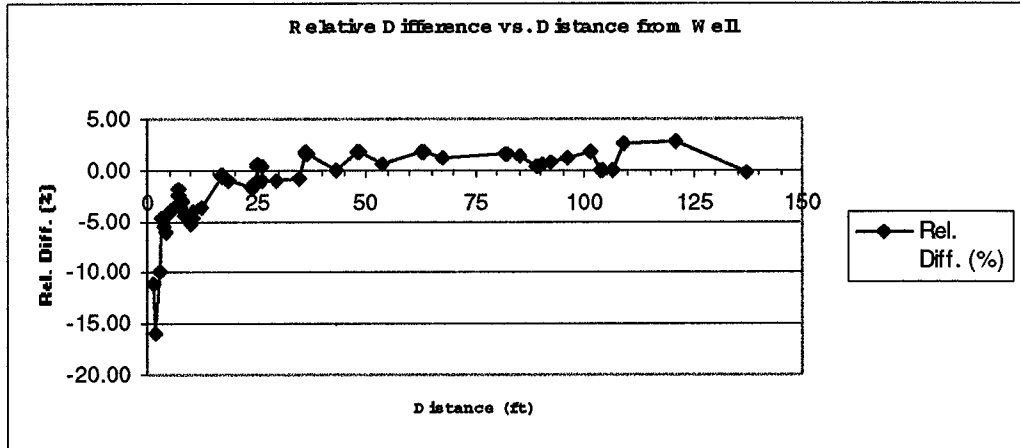


Figure B11-2. Relative differentials versus distance from well in verification plane.

Table B11-4. Evaluation of symmetry of results from DYNFLOW.

node	71	8	78	bench mark
[level]	[11]	[11]	[1]	
head	0.37	0.36	0.36	0.36
node	4	31	34	bench mark
[level]	[7]	[7]	[1]	
head	1.29	1.35	1.30	1.32
node	2	11	12	bench mark
[level]	[4]	[4]	[1]	
head	4.19	4.54	4.42	4.50
node	102	103	101	bench mark
[level]	[3]	[3]	[1]	
head	8.69	8.11	8.58	9.00
node	115	114	113	bench mark
[level]	[2]	[2]	[1]	
head	21.89	16.82	20.02	22.51

Performance notes: For short distance from the well (less than 15 ft), the comparison between DYNFLOW and the bench mark show increased values for the relative differentials (see Fig. B11-2). The accuracy can be improved by refining the grid near the well. Also, an asymmetrical effect appears to occur near the domain boundaries, especially in the vicinity of the well where gradients are highest. Reducing the tolerance an order of magnitude does not make a difference, only grid refining does. Selection of solver type may also affect this behavior.

Command File DFTC11.CFI

```
OUTPUT DFTC11.OUT
!
TITLE
DYNFLOW VERIFICATION CASE NO. 11 -- SPHERICAL FLOW
STEADY STATE FLOW TOWARDS A CONSTANT PUMPING POINT
RUN BY PVDH, BOULDER, CO
!
! READ IN GRID
GRID read DFTC11.GRF form
!
! SPECIFY NUMBER OF LEVELS AND LEVEL ELEVATIONS
LEVE 13.
ELEV 0.0 level 1
ELEV 1.0 LEVEL 2
ELEV 2.5 LEVEL 3
ELEV 5.0 LEVEL 4
ELEV 7.5 LEVEL 5
ELEV 10. LEVEL 6
ELEV 17. LEVEL 7
ELEV 25. LEVEL 8
ELEV 36. LEVEL 9
ELEV 48. LEVEL 10
ELEV 63. LEVEL 11
ELEV 82. LEVEL 12
ELEV 104. LEVEL 13
!
! SPECIFY PROPERTIES AND ASSIGN TO LAYERS
PROP
1,1.0,1.0,1.0,0,0,0
ELEM 301. LAYER ALL ELEM ALL
!
! READ IN STARTING HEADS
head read DFTC11.HDS
HEAD 1000. ADD
!
! ASSIGN BOUNDARY CONDITIONS
FREE
! FIX LATERAL BOUNDARY NODES
fix level all node &
10,20,30,40,50,60,70,80,90,100, &
91,92,93,94,95,96,97,98,99,
! FIX HEADS AT TOP OF MODEL
fix level 13 node all
! ASSIGN PUMPING
flux 50. level 1 node 1
!
DT
TOL .001
ALPHA 1.5
ITIN 50.
ITER 30.
GOTIL
```

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

```
! CHECK SIMULATED HEADS AGAINST STARTING HEADS
head 1000. subt
head show node 113
head show node 101
head show node 12
head show node 34
head show node 78
end
```

Grid File DFTC11.GRF

115	186				
1	0.000	0.000	56	36.00	36.00
2	0.000	5.000	57	36.00	48.00
3	0.000	10.00	58	36.00	63.00
4	0.000	17.00	59	36.00	82.00
5	0.000	25.00	60	36.00	104.0
6	0.000	36.00	61	48.00	0.000
7	0.000	48.00	62	48.00	5.000
8	0.000	63.00	63	48.00	10.00
9	0.000	82.00	64	48.00	17.00
10	0.000	104.0	65	48.00	25.00
11	5.000	0.000	66	48.00	36.00
12	5.000	5.000	67	48.00	48.00
13	5.000	10.00	68	48.00	63.00
14	5.000	17.00	69	48.00	82.00
15	5.000	25.00	70	48.00	104.0
16	5.000	36.00	71	63.00	0.000
17	5.000	48.00	72	63.00	5.000
18	5.000	63.00	73	63.00	10.00
19	5.000	82.00	74	63.00	17.00
20	5.000	104.0	75	63.00	25.00
21	10.00	0.000	76	63.00	36.00
22	10.00	5.000	77	63.00	48.00
23	10.00	10.00	78	63.00	63.00
24	10.00	17.00	79	63.00	82.00
25	10.00	25.00	80	63.00	104.0
26	10.00	36.00	81	82.00	0.000
27	10.00	48.00	82	82.00	5.000
28	10.00	63.00	83	82.00	10.00
29	10.00	82.00	84	82.00	17.00
30	10.00	104.0	85	82.00	25.00
31	17.00	0.000	86	82.00	36.00
32	17.00	5.000	87	82.00	48.00
33	17.00	10.00	88	82.00	63.00
34	17.00	17.00	89	82.00	82.00
35	17.00	25.00	90	82.00	104.0
36	17.00	36.00	91	104.0	0.000
37	17.00	48.00	92	104.0	5.000
38	17.00	63.00	93	104.0	10.00
39	17.00	82.00	94	104.0	17.00
40	17.00	104.0	95	104.0	25.00
41	25.00	0.000	96	104.0	36.00
42	25.00	5.000	97	104.0	48.00
43	25.00	10.00	98	104.0	63.00
44	25.00	17.00	99	104.0	82.00
45	25.00	25.00	100	104.0	104.0
46	25.00	36.00	101	2.500	2.500
47	25.00	48.00	102	2.500	0.000
48	25.00	63.00	103	0.000	2.500
49	25.00	82.00	104	7.500	2.500
50	25.00	104.0	105	7.500	7.500
51	36.00	0.000	106	2.500	7.500
52	36.00	5.000	107	7.500	0.000
53	36.00	10.00	108	0.000	7.500
54	36.00	17.00	109	2.500	5.000
55	36.00	25.00	110	5.000	2.500

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

111	5.000	7.500	2	12	109	101
112	7.500	5.000	4	13	3	106
113	1.000	1.000	5	3	14	4
114	0.000	1.000	6	3	13	14
115	1.000	0.000	7	4	15	5
			8	4	14	15
			9	5	16	6
			10	5	15	16
			11	6	17	7
			12	6	16	17
			13	7	18	8
			14	7	17	18
			15	8	19	9
			16	8	18	19
			17	9	20	10
			18	9	19	20
			19	104	12	110
			20	22	112	104
			21	105	13	111
			22	23	13	105
			23	13	23	14
			24	23	24	14
			25	14	24	15
			26	24	25	15
			27	15	25	16
			28	25	26	16
			29	16	26	17
			30	26	27	17
			31	17	27	18
			32	27	28	18
			33	18	28	19
			34	28	29	19
			35	19	29	20
			36	29	30	20
			37	21	32	22
			38	21	31	32
			39	22	33	23
			40	22	32	33
			41	23	34	24
			42	23	33	34
			43	24	35	25
			44	24	34	35
			45	25	36	26
			46	25	35	36
			47	26	37	27
			48	26	36	37
			49	27	38	28
			50	27	37	38
			51	28	39	29
			52	28	38	39
			53	29	40	30
			54	29	39	40
			55	31	41	32
			56	41	42	32
			57	32	42	33
			58	42	43	33
			59	33	43	34
			60	43	44	34
			61	34	44	35
			62	44	45	35
			63	35	45	36
			64	45	46	36
			65	36	46	37
			66	46	47	37
			67	37	47	38
			68	47	48	38
			69	38	48	39
			70	48	49	39
			71	39	49	40
			72	49	50	40

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

73	41	52	42	143	79	89	80
74	41	51	52	144	89	90	80
75	42	53	43	145	81	92	82
76	42	52	53	146	81	91	92
77	43	54	44	147	82	93	83
78	43	53	54	148	82	92	93
79	44	55	45	149	83	94	84
80	44	54	55	150	83	93	94
81	45	56	46	151	84	95	85
82	45	55	56	152	84	94	95
83	46	57	47	153	85	96	86
84	46	56	57	154	85	95	96
85	47	58	48	155	86	97	87
86	47	57	58	156	86	96	97
87	48	59	49	157	87	98	88
88	48	58	59	158	87	97	98
89	49	60	50	159	88	99	89
90	49	59	60	160	88	98	99
91	51	61	52	161	89	100	90
92	61	62	52	162	89	99	100
93	52	62	53	164	11	110	101
94	62	63	53	165	101	113	102
95	53	63	54	166	11	101	102
96	63	64	54	168	2	103	101
97	54	64	55	170	21	22	104
98	64	65	55	171	105	12	112
99	55	65	56	172	22	23	105
100	65	66	56	173	106	2	109
101	56	66	57	174	12	111	106
102	66	67	57	175	104	11	107
103	57	67	58	176	21	104	107
104	67	68	58	177	108	2	106
105	58	68	59	178	3	108	106
106	68	69	59	179	12	106	109
107	59	69	60	180	2	101	109
108	69	70	60	181	11	104	110
109	61	72	62	182	12	101	110
110	61	71	72	183	12	105	111
111	62	73	63	184	13	106	111
112	62	72	73	185	22	105	112
113	63	74	64	186	12	104	112
114	63	73	74	187	101	103	113
115	64	75	65	188	113	1	115
116	64	74	75	189	113	115	102
117	65	76	66	190	114	1	113
118	65	75	76	191	103	114	113
119	66	77	67				
120	66	76	77				
121	67	78	68				
122	67	77	78				
123	68	79	69				
124	68	78	79				
125	69	80	70				
126	69	79	80				
127	71	81	72				
128	81	82	72				
129	72	82	73				
130	82	83	73				
131	73	83	74				
132	83	84	74				
133	74	84	75				
134	84	85	75				
135	75	85	76				
136	85	86	76				
137	76	86	77				
138	86	87	77				
139	77	87	78				
140	87	88	78				
141	78	88	79				
142	88	89	79				

Results File DFTC11.OUT (reformatted)

```
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```

```
--- THIS RUN OF DYNFLOW5
--- VERSION 5.18E (ICCG SOLVER) SEPT. 1996
--- STARTED AT 7/15/99 11:56:20.
--- ALL OUTPUT WILL BE TAGGED WITH THIS DATE/TIME.
```

```
!
TITLE
```

```
ENTER THREE TITLE LINES ("!" FOR NO CHANGE) :
TITLE LINE #1 :
TITLE LINE #2 :
TITLE LINE #3 :
```

```
HERE IS HOW THE TITLE LINES LOOK --
DYNFLOW VERIFICATION CASE NO. 11 -- SPHERICAL FLOW
STEADY STATE FLOW TOWARDS A CONSTANT PUMPING POINT
RUN BY PVDH, BOULDER, CO
```

```
!
! READ IN GRID
GRID read DFTC11.GRF form
```

```
GRID INFO. NOW BEING PROCESSED ...
```

```
--- GRID DATA NOW BEING CLEARED,
--- PRIOR TO FILE READING.
```

```
*** ERRORS OR WARNINGS DURING PROCESSING OF
*** ELEMENT GRID INFORMATION.
*** A VALUE OF -9999 INDICATES THAT THE PARAMETER
*** IS NOT A PROBLEM, OR IT WAS NOT COMPUTED,
*** POSSIBLY DUE TO A PREVIOUS PROBLEM.
*** NOTE THE FOLLOWING:
*** MAX. EXTERNAL ELEMENT NO. .... : 10000
*** MAX. EXTERNAL NODE NO. .... : *****
*** ELEM SIDE RATIO WARNING LIMIT ... : 3.000
*** ELEM SIDE RATIO ERROR LIMIT ... : 6.000
*** ALSO, ELEMENT AREA FOR NON 1-D ELEMENTS
*** IS NOT ALLOWED TO BE ZERO OR BELOW.
*** LIST OF PARAMETERS FOR ELEMENTS WITH WARNINGS
*** AND/OR ERRORS IS AS FOLLOWS :
```

*** WARNING, *** ERROR, *** 1D ELEM	ELEM NOS ON FILE		EXT ELEM NO -----		CONNECTED NODES -----			SIDE LENGTH RATIOS -----			HORIZONTAL SURFACE AREA
	INT	EXT	BEYOND RANGE	IN USE AT INT	NODE #1	NODE #2	NODE #3	SIDE 1-3/2-3	SIDE 1-2/1-3	SIDE 1-2/2-3	
*** WARNING	11	13								3.16	
*** WARNING	12	14							3.16		
*** WARNING	13	15						3.80		3.93	
*** WARNING	14	16							3.93	3.80	
*** WARNING	15	17						4.40		4.51	
*** WARNING	16	18							4.51	4.40	
*** WARNING	29	31								3.16	
*** WARNING	30	32						3.16			

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

*** WARNING	31	33		3.80	3.93
*** WARNING	32	34	3.93		3.80
*** WARNING	33	35		4.40	4.51
*** WARNING	34	36	4.51		4.40
*** WARNING	51	53	3.14		3.30
*** WARNING	52	54		3.30	3.14
*** WARNING	107	109		3.16	
*** WARNING	108	110	3.16		
*** WARNING	109	111		3.16	
*** WARNING	110	112	3.16		
*** WARNING	125	127	3.93	3.80	
*** WARNING	126	128		3.93	3.80
*** WARNING	127	129	3.93	3.80	
*** WARNING	128	130		3.93	3.80
*** WARNING	143	145	4.40	4.51	
*** WARNING	144	146	4.51		4.40
*** WARNING	145	147	4.40	4.51	
*** WARNING	146	148	4.51		4.40
*** WARNING	147	149	3.14	3.30	
*** WARNING	148	150	3.30		3.14

--- GRID WAS PROCESSED OK.

*** WRITING GRID DATA TO FILE "GRID.SUM"

```

!
! SPECIFY NUMBER OF LEVELS AND LEVEL ELEVATIONS
LEVE 13.
ELEV 0.0 level 1
ELEV 1.0 LEVEL 2
ELEV 2.5 LEVEL 3
ELEV 5.0 LEVEL 4
ELEV 7.5 LEVEL 5
ELEV 10. LEVEL 6
ELEV 17. LEVEL 7
ELEV 25. LEVEL 8
ELEV 36. LEVEL 9
ELEV 48. LEVEL 10
ELEV 63. LEVEL 11
ELEV 82. LEVEL 12
ELEV 104. LEVEL 13
!
! SPECIFY PROPERTIES AND ASSIGN TO LAYERS
PROP

ENTER PROPERTIES IN FOLLOWING ORDER --
#,KXX,KYY,KZZ,STORATIVITY,SPEC. YIELD,RECHARGE,THETA,PHI,SPEC. GRAV.,EFF. STRESS:
1,1.0,1.0,1.0,0,0,0
ELEM 301. LAYER ALL ELEM ALL
!
! READ IN STARTING HEADS
head read DFTC11.HDS
HEAD 1000. ADD
!
! ASSIGN BOUNDARY CONDITIONS
FREE
! FIX LATERAL BOUNDARY NODES
fix level all node &
10,20,30,40,50,60,70,80,90,100, &
91,92,93,94,95,96,97,98,99,
! FIX HEADS AT TOP OF MODEL
fix level 13 node all
! ASSIGN PUMPING
flux 50. level 1 node 1
!
DT
TOL .0001
ALPHA 1.5
ITIN 50.
ITER 30.

```

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

GOTIL

*** I.C.C.G. SOLVER SELECTED

CG HAS CONVERGED IN 37 ITERATIONS
 1***** DYNFLOW5 ***** VERSION 5.18E (ICCG SOLVER) SEPT. 1996
 ***** DYNFLOW5 *****
 *****.....DYNFLOW VERIFICATION CASE NO. 11 -- SPHERICAL FLOW
 RUN DATE : 7/15/99 *****
 *****.....STEADY STATE FLOW TOWARDS A CONSTANT PUMPING POINT
 RUN TIME : 11:56:20 *****
 *****.....RUN BY PVDH, BOULDER, CO *****

 *** ALL 13 LEVELS *** TIME .00000 ** TIME STEP 1 *** TIME STEP DURATION .00000 ***
 0 ITERATIONS (30 MAX) *****

MODEL SIZE

=====

NODES	ELEMENTS	LEVELS	LAYERS
115	186	13	12

PROPERTY SETS

=====

----- HYDRAULIC CONDUCTIVITIES -----				SPECIFIC			
NO.	HORIZONTAL-X	HORIZONTAL-Y	VERTICAL-Z	STORATIVITY	YIELD	RECHARGE	THETA
1	1.0000000	1.0000000	1.0000000	.0000E+00	.000000	.00000000	.00

PH	UNIT	EFFECT.	# OF
---	---	---	---
	WEIGHT	STRESS	ELEMS NO.
.00	.0000	.0000	2232 1

 * CONDITIONAL BOUNDARIES (See Node List Table Above) *

NO. OF INVOKED RISING WATER NODES: 0
 NO. OF INVOKED DRY NODES: 0
 NO. OF RISING WATER NODES BELOW TOP OF MODEL (CODE = *RISI*) 0

***** DYNFLOW5 ***** VERSION 5.18E (ICCG SOLVER) SEPT. 1996
 *****.....DYNFLOW VERIFICATION CASE NO. 11 -- SPHERICAL FLOW
 RUN DATE : 7/15/99 *****
 *****.....STEADY STATE FLOW TOWARDS A CONSTANT PUMPING POINT
 RUN TIME : 11:56:20 *****
 *****.....RUN BY PVDH, BOULDER, CO *****

 *** ALL 13 LEVELS *** TIME .00000 ** TIME STEP 1 *** TIME STEP DURATION .00000
 *** 0 ITERATIONS (30 MAX) *****

MASS BALANCE TABLE

=====

FOR THE ENTIRE SYSTEM OF 13 LEVELS AND 115 NODES

=====

LEVEL NO.	TOTAL FLUX	FIXED HEAD FLUX	PUMPING FLUX	RECHARGE FLUX	STORAGE FLUX
-----------	------------	-----------------	--------------	---------------	--------------

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

INFLOWS	1	50.000	.000	50.000	.000	.000
	2	.001	.001	.000	.000	.000
	3	.046	.046	.000	.000	.000
	4	.000	.000	.000	.000	.000
	5	.000	.000	.000	.000	.000
	6	.000	.000	.000	.000	.000
	7	.000	.000	.000	.000	.000
	8	.332	.332	.000	.000	.000
	9	2.059	2.059	.000	.000	.000
	10	5.725	5.725	.000	.000	.000
	11	13.940	13.940	.000	.000	.000
	12	34.458	34.458	.000	.000	.000
	13	.856	.856	.000	.000	.000
	TOTAL	107.420	57.420	50.000	.000	.000
OUTFLOWS	1	-.211	-.211	.000	.000	.000
	2	-.613	-.613	.000	.000	.000
	3	-.598	-.598	.000	.000	.000
	4	-1.576	-1.576	.000	.000	.000
	5	-1.268	-1.268	.000	.000	.000
	6	-2.083	-2.083	.000	.000	.000
	7	-3.571	-3.571	.000	.000	.000
	8	-4.589	-4.589	.000	.000	.000
	9	-6.697	-6.697	.000	.000	.000
	10	-10.345	-10.345	.000	.000	.000
	11	-18.369	-18.369	.000	.000	.000
	12	-31.669	-31.669	.000	.000	.000
	13	-25.830	-25.830	.000	.000	.000
	TOTAL	-107.420	-107.420	.000	.000	.000
NET FLUX	1	49.789	-.211	50.000	.000	.000
	2	-.612	-.612	.000	.000	.000
	3	-.552	-.552	.000	.000	.000
	4	-1.575	-1.575	.000	.000	.000
	5	-1.268	-1.268	.000	.000	.000
	6	-2.083	-2.083	.000	.000	.000
	7	-3.571	-3.571	.000	.000	.000
	8	-4.257	-4.257	.000	.000	.000
	9	-4.638	-4.638	.000	.000	.000
	10	-4.620	-4.620	.000	.000	.000
	11	-4.429	-4.429	.000	.000	.000
	12	2.789	2.789	.000	.000	.000
	13	-24.974	-24.974	.000	.000	.000
	TOTAL	.000	-50.000	50.000	.000	.000

MASS BALANCE ERROR .00 %

REJECTED RECHARGE, NODAL POINT BASIS : .00
 REJECTED RECHARGE, ELEMENT BASIS : .00

***** DYNFLOW5 ***** VERSION 5.18E (ICCG SOLVER) SEPT. 1996
 ***** DYNFLOW VERIFICATION CASE NO. 11 -- SPHERICAL FLOW
 RUN DATE : 7/15/99 *****
 ***** STEADY STATE FLOW TOWARDS A CONSTANT PUMPING POINT
 RUN TIME : 11:56:20 *****
 ***** RUN BY PVDH, BOULDER, CO

 *** ALL 13 LEVELS *** TIME .00000 ** TIME STEP 1 *** TIME STEP DURATION .00000
 *** 0 ITERATIONS (30 MAX) *****

SELECTED UNIT-SPECIFIC INFORMATION, ASSUMING FEET-DAY UNITS USED IN MODEL

=====

GRID AREA = 10816.0 (SQ. FT.)
 = .000 (SQ. MI.)

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

```

NET TOTAL ELEMENT RECHARGE = .00000 (FT./DAY)
                          = .00 (INCHES/YEAR)
                          = .000 (MGD/SQ.MI.)

FIXED HEAD DISCHARGE (OUTFLOW) = -.00462 (FT./DAY)
                              = -20.25 (INCHES/YEAR)
                              = -.964 (MGD/SQ.MI.)

FIXED HEAD RECHARGE ( INFLOW) = .0007 (CFS)
FIXED HEAD DISCHARGE (OUTFLOW) = -.0012 (CFS)
FIXED HEAD FLUX (NET INFLOW) = -.0006 (CFS)

PUMPING RECHARGE ( INFLOW) = .3 (GPM)
PUMPING DISCHARGE (OUTFLOW) = .0 (GPM)
PUMPING FLUX (NET INFLOW) = .3 (GPM)

PUMPING RECHARGE ( INFLOW) = .000 (MGD)
PUMPING DISCHARGE (OUTFLOW) = .000 (MGD)
PUMPING FLUX (NET INFLOW) = .000 (MGD)
    
```

VOLUMES
=====

LAYER	MEDIA	WATER
1	10816.020	.000
2	16224.000	.000
3	27039.960	.000
4	27039.960	.000
5	27039.960	.000
6	75711.810	.000
7	86528.130	.000
8	118976.200	.000
9	129792.000	.000
10	162240.000	.000
11	205504.300	.000
12	237952.300	.000
TOTAL	1124865.000	.000

! CHECK SIMULATED HEADS AGAINST STARTING HEADS
head 1000. subt

head show node 113

1,	113,	20.02307	, .0000000
2,	113,	15.45276	, .0000000
3,	113,	9.980225	, .0000000
4,	113,	5.873169	, .0000000
5,	113,	4.020508	, .0000000
6,	113,	2.986694	, .0000000
7,	113,	1.852722	, .0000000
8,	113,	1.276489	, .0000000
9,	113,	.8978271	, .0000000
10,	113,	.6742554	, .0000000
11,	113,	.5141602	, .0000000
12,	113,	.3944092	, .0000000
13,	113,	.3060303	, .0000000

head show node 101

1,	101,	8.577759	, .0000000
2,	101,	8.185730	, .0000000
3,	101,	6.908691	, .0000000
4,	101,	5.003235	, .0000000
5,	101,	3.721069	, .0000000
6,	101,	2.859131	, .0000000
7,	101,	1.826172	, .0000000
8,	101,	1.267700	, .0000000
9,	101,	.8949585	, .0000000
10,	101,	.6729736	, .0000000
11,	101,	.5136108	, .0000000
12,	101,	.3941650	, .0000000
13,	101,	.3060303	, .0000000

head show node 12

1,	12,	4.416321	, .0000000
2,	12,	4.345276	, .0000000
3,	12,	4.105103	, .0000000
4,	12,	3.512207	, .0000000
5,	12,	2.960571	, .0000000
6,	12,	2.505432	, .0000000
7,	12,	1.711792	, .0000000
8,	12,	1.228394	, .0000000

9,	12,	.8805542	, .0000000
10,	12,	.6671143	, .0000000
11,	12,	.5110474	, .0000000
12,	12,	.3930054	, .0000000
13,	12,	.3049927	, .0000000

head show node 34

1,	34,	1.301270	,	.0000000
2,	34,	1.300964	,	.0000000
3,	34,	1.296509	,	.0000000
4,	34,	1.278992	,	.0000000
5,	34,	1.249084	,	.0000000
6,	34,	1.209900	,	.0000000
7,	34,	1.068787	,	.0000000
8,	34,	.9102783	,	.0000000
9,	34,	.7348022	,	.0000000
10,	34,	.5963745	,	.0000000
11,	34,	.4772339	,	.0000000
12,	34,	.3773804	,	.0000000
13,	34,	.2979736	,	.0000000

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

head show node 78

1,	78,	.3585815	,	.0000000
2,	78,	.3585815	,	.0000000
3,	78,	.3584595	,	.0000000
4,	78,	.3580933	,	.0000000
5,	78,	.3574829	,	.0000000
6,	78,	.3566284	,	.0000000
7,	78,	.3530884	,	.0000000
8,	78,	.3469238	,	.0000000
9,	78,	.3354492	,	.0000000
10,	78,	.3202515	,	.0000000
11,	78,	.2990112	,	.0000000
12,	78,	.2703247	,	.0000000
13,	78,	.2319946	,	.0000000

head show node 2

1,	2,	6.119263	,	.0000000
2,	2,	5.958923	,	.0000000
3,	2,	5.342651	,	.0000000
4,	2,	4.190125	,	.0000000
5,	2,	3.317444	,	.0000000
6,	2,	2.685974	,	.0000000
7,	2,	1.760681	,	.0000000
8,	2,	1.243225	,	.0000000
9,	2,	.8847656	,	.0000000
10,	2,	.6687622	,	.0000000
11,	2,	.5117798	,	.0000000
12,	2,	.3933105	,	.0000000
13,	2,	.3060303	,	.0000000

head show node 4

1,	4,	1.877563	,	.0000000
2,	4,	1.869629	,	.0000000
3,	4,	1.844543	,	.0000000
4,	4,	1.772156	,	.0000000
5,	4,	1.683533	,	.0000000
6,	4,	1.587280	,	.0000000
7,	4,	1.291626	,	.0000000
8,	4,	1.035095	,	.0000000
9,	4,	.7951660	,	.0000000
10,	4,	.6270752	,	.0000000
11,	4,	.4920654	,	.0000000
12,	4,	.3839722	,	.0000000
13,	4,	.3020020	,	.0000000

head show node 8

1,	8,	.5110474	,	.0000000
2,	8,	.5108643	,	.0000000
3,	8,	.5103149	,	.0000000
4,	8,	.5087891	,	.0000000
5,	8,	.5067139	,	.0000000
6,	8,	.5041504	,	.0000000
7,	8,	.4913940	,	.0000000
8,	8,	.4726563	,	.0000000
9,	8,	.4412842	,	.0000000
10,	8,	.4055786	,	.0000000
11,	8,	.3622437	,	.0000000
12,	8,	.3127441	,	.0000000
13,	8,	.2620239	,	.0000000

head show node 11

1,	11,	6.326294	,	.0000000
2,	11,	6.247803	,	.0000000
3,	11,	5.684875	,	.0000000
4,	11,	4.544189	,	.0000000
5,	11,	3.537964	,	.0000000
6,	11,	2.779724	,	.0000000
7,	11,	1.828979	,	.0000000
8,	11,	1.271790	,	.0000000
9,	11,	.8977051	,	.0000000
10,	11,	.6739502	,	.0000000
11,	11,	.5140381	,	.0000000
12,	11,	.3944092	,	.0000000
13,	11,	.3060303	,	.0000000

head show node 31

1,	31,	1.879150	,	.0000000
2,	31,	1.878113	,	.0000000
3,	31,	1.864075	,	.0000000
4,	31,	1.812073	,	.0000000
5,	31,	1.728943	,	.0000000
6,	31,	1.628296	,	.0000000
7,	31,	1.350037	,	.0000000
8,	31,	1.077148	,	.0000000
9,	31,	.8246460	,	.0000000
10,	31,	.6431885	,	.0000000
11,	31,	.5012207	,	.0000000
12,	31,	.3892822	,	.0000000
13,	31,	.3020020	,	.0000000

head show node 71

1,	71,	.5115967	,	.0000000
2,	71,	.5115356	,	.0000000
3,	71,	.5113525	,	.0000000
4,	71,	.5105591	,	.0000000
5,	71,	.5089722	,	.0000000
6,	71,	.5066528	,	.0000000
7,	71,	.4975586	,	.0000000
8,	71,	.4810791	,	.0000000
9,	71,	.4518433	,	.0000000
10,	71,	.4163818	,	.0000000
11,	71,	.3738403	,	.0000000
12,	71,	.3265381	,	.0000000
13,	71,	.2620239	,	.0000000

head show node 102

1,	102,	13.47302	,	.0000000
2,	102,	11.79132	,	.0000000
3,	102,	8.692627	,	.0000000
4,	102,	5.586609	,	.0000000
5,	102,	3.936829	,	.0000000
6,	102,	2.951538	,	.0000000
7,	102,	1.850891	,	.0000000
8,	102,	1.276550	,	.0000000
9,	102,	.8982544	,	.0000000
10,	102,	.6743164	,	.0000000
11,	102,	.5142212	,	.0000000
12,	102,	.3944092	,	.0000000
13,	102,	.3060303	,	.0000000

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

head show node 103

1,	103,	12.25073	,	.0000000
2,	103,	10.75549	,	.0000000
3,	103,	8.112427	,	.0000000
4,	103,	5.291199	,	.0000000
5,	103,	3.813599	,	.0000000
6,	103,	2.918579	,	.0000000
7,	103,	1.829834	,	.0000000
8,	103,	1.268616	,	.0000000
9,	103,	.8947144	,	.0000000
10,	103,	.6729736	,	.0000000
11,	103,	.5136108	,	.0000000
12,	103,	.3941040	,	.0000000
13,	103,	.3060303	,	.0000000

head show node 114

1,	114,	25.70203	,	.0000000
2,	114,	16.81525	,	.0000000
3,	114,	10.09344	,	.0000000
4,	114,	5.845642	,	.0000000
5,	114,	4.014282	,	.0000000
6,	114,	2.989990	,	.0000000
7,	114,	1.851685	,	.0000000
8,	114,	1.276245	,	.0000000
9,	114,	.8977051	,	.0000000
10,	114,	.6741943	,	.0000000
11,	114,	.5141602	,	.0000000
12,	114,	.3943481	,	.0000000
13,	114,	.3060303	,	.0000000

head show node 115

1,	115,	36.82239	,	.0000000
2,	115,	21.88696	,	.0000000
3,	115,	11.60376	,	.0000000
4,	115,	6.235718	,	.0000000
5,	115,	4.117554	,	.0000000
6,	115,	3.010010	,	.0000000
7,	115,	1.858826	,	.0000000
8,	115,	1.278381	,	.0000000
9,	115,	.8984985	,	.0000000
10,	115,	.6744995	,	.0000000
11,	115,	.5142822	,	.0000000
12,	115,	.3944092	,	.0000000
13,	115,	.3060303	,	.0000000

end

* CONDITIONAL BOUNDARIES (See Node List Table Above) *

NO. OF INVOKED RISING WATER NODES:	0
NO. OF INVOKED DRY NODES:	0
NO. OF RISING WATER NODES BELOW TOP OF MODEL (CODE = *RISI*)	0

--- THIS RUN OF DYNFLOW5
--- VERSION 5.18E (ICCG SOLVER) SEPT. 1996
--- WHICH STARTED AT 7/15/99 11:56:20

--- IS ENDING AT 7/15/99 11:56:25

Test 12 EVAPORATION AND RECHARGE

test #: DFTC12

command file: DFTC12.CFI

grid file: DFTC12.GRF

output file: DFTC12.OUT

date of test execution: January 7, 1998

title: Evaporation and recharge computation.

description: Steady state horizontal uniform flow between two fully penetrating drains in an isotropic, homogeneous, unconfined aquifer with a horizontal impermeable base (see figure B12-1) modified by recharge and evaporation. The geometry of this model is identical to that of DFTC01A except for the height.

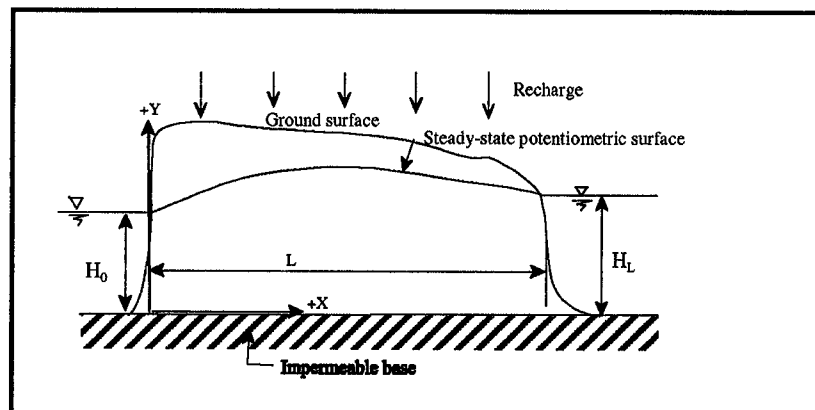


Figure B12-1. Conceptual model for test case DFTC12.

tested functions: Distributed recharge and evaporation algorithms and their additive use.

model domain: Strip between two parallel drains with a length, width and thickness of $L=100$ ft, $w=10$ ft, and $b=80$ ft respectively.

boundary conditions: Constant head at left boundary ($x=0$ ft) is $H_0=50$ ft; constant head at right boundary ($x=100$ ft) is $H_L=75$ ft; no-flow boundaries in y - and z -direction; free surface on top, no flow boundary at bottom.

benchmark: The test strategy is two-fold: 1) compare recharge results with analytical solution; 2) check global water balance and local evaporation "by hand".

The analytical solution for uniform recharge is:

$$K (h^2 - H_0^2) - Nx (L - x) + Kx/L (H_0^2 - H_L^2) = 0 \text{ (Bear, 1979)}$$

in which N is the uniform recharge rate [L/T] and K the hydraulic conductivity [L/T].

grid: Two-dimensional horizontal grid with one layer and two levels; in plan view, the grid consists of 33 nodes in three rows, defining 40 elements (see figure B12-2).

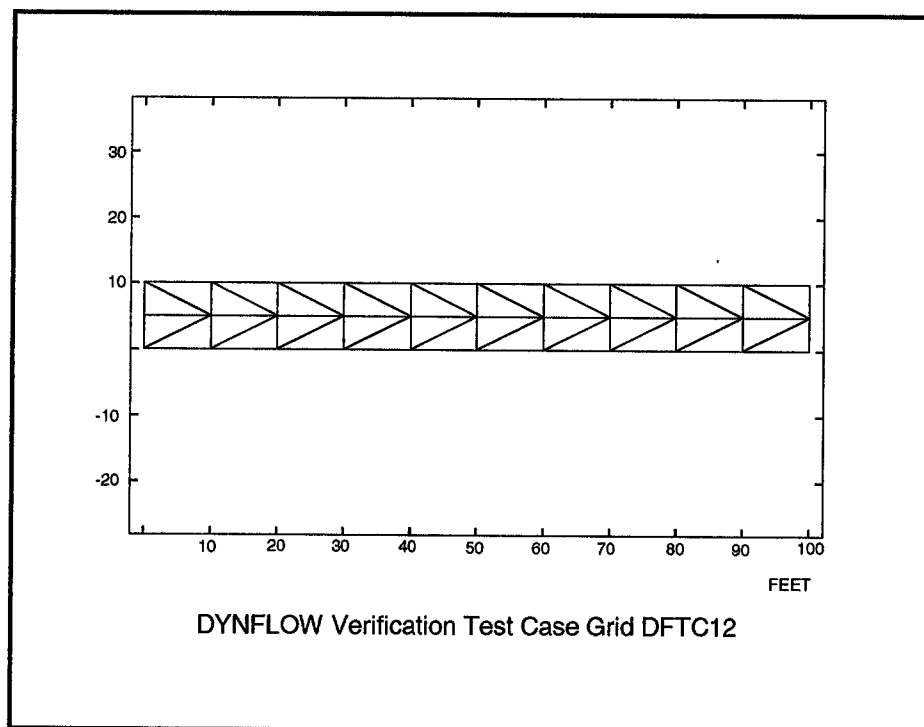


Figure B12-2.

initial conditions: n.a. (steady state)

time-stepping: n.a. (steady state)

system parameters: Hydraulic conductivity $K_x = K_y = 1.0$ ft/d; $K_z = 100.0$ ft/d

control parameters: Tolerance = 1.0 ft

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Tabular listing of heads (see Table B12-1) and manual water budget calculations (see listing of DFTC12.OUT); no statistical measures calculated.

Table B12-1. Results for recharge $N = 0.01$ ft/d.

distance from origin (ft)	piezometric head (ft) for average of all nodes at specified distance (see DFTC12-results.xls for details)	
	analytical solution	DYNFLOW
0	50.00	50.00
10	53.12	53.12
20	56.04	56.05
30	58.81	58.81
40	61.43	61.44
50	63.93	63.94
60	66.32	66.33
70	68.62	68.62
80	70.82	70.83
90	72.95	72.95
100	75.00	175.00
total mass balance error	.0006%	

Notes on performance: The comparison for the head is excellent and the water balance calculations show that the relevant algorithms function correctly.

Command File DFTC12.CFI

OUTPUT DFTC12.OUT

TITLE
VERIFICATION CASE NO. 12 -- EVAPORATION AND RECHARGE
STEADY-STATE UNCONFINED ONE-DIMENSIONAL FLOW WITH RECHARGE AND EVAPORATION
RUN BY PVDH, BOULDER CO.

TEXT

Test Summary: DYNFLOW - Version 5.18 (September 1996; Built June 4, 1997)

THIS TEST CASE DEMONSTRATES THAT APPROPRIATE
RECHARGE AND EVAPORATION TERMS ARE ADDED TO THE SOLUTION.

ENDT

! INITIAL MODEL SETUP

GRID READ DFTC12.GRF FORMatted
LEVEL 2.
FREE
ELEM 301.
ELEV 0. LEVELSING 1
ELEV 80. LEVELSING 2
INIT 75.
INIT 50. LEVELALL NODE RANGE 1 3
FIX LEVELALL NODE RANGE 1 3
FIX LEVELALL NODE RANGE 31 33
DT 0.
TOL 0001.
ITER 100.
PRAL
NOPR

! THE FIRST CASE SETS THE ELEMENT RECHARGE
! USING THE ORIGINAL METHOD OF ASSIGNMENT
! OF RECHARGE THROUGH THE 'PROP' COMMAND.
! THE GRID AREA IS 1000 FT.SQ. AND THE
! RECHARGE IS 0.01 FT/D GIVING A TOTAL FLUX
! OF 10 CFD

PROP
1,1.,1.,100.,0.,0.,0.01,0.,0.

GOTIL 0.

PRIN

! THE COMPUTED TOTAL RECHARGE SHOWN BELOW SHOULD BE 10 CFD (FIRST LINE).

FLXL LEVEL 2 SUM

! THE SECOND CASE USES THE 'RECH' COMMAND WITH
! THE ELEMENT SPECIFICATION. THIS RECHARGE WILL BE ADDED
! TO THE ORIGINAL VALUE GIVEN THROUGH THE 'PROP' COMMAND
! TO GIVE A TOTAL OF 20 CFD.

RECH 0.01 ELEM ALL
GOTI 0.

PRIN

! THE COMPUTED TOTAL RECHARGE SHOWN BELOW SHOULD BE 20 CFD (FIRST LINE).

FLXL LEVEL 2 SUM

! THE THIRD CASE USES THE 'RECH' COMMAND WITH
! THE NODE SPECIFICATION. THE NUMBER OF NODES IS
! 33 SO THAT THE TOTAL RECHARGE WILL NOW BE
! 43 CFD (10 + 33*1.0). NOTE THAT THE 'RECH' COMMAND
! REPLACES THE ORIGINAL VALUES SET BY THE 'RECH'
! COMMAND BEFORE.

RECH 1.0 NODE ALL
GOTI 0.

PRIN

! THE COMPUTED TOTAL RECHARGE SHOWN BELOW SHOULD BE 43 CFD (FIRST LINE).

Test Summary: DYNFLOW - Version 5.18 (September 1996; Built June 4, 1997)

FLXL LEVEL 2 SUM

! THE FOURTH CASE USES THE 'EVAP' COMMAND FOR ALL NODES.
! TO CHECK ITS OPERATION, THE RECHARGE AT NODE 29 (LOCATED ALONG THE
! CENTER AXIS OF THE GRID) WITH NO EVAPORATION SHOULD BE 1.5 CFD.
! THE RECHARGE AT NODE 29 WITH EVAPORATION SHOULD BE 1.3424 CFD.

! RECHARGE AT NODE 29 WITHOUT EVAPORATION SHOWN BELOW SHOULD EQUAL 1.5 CFD.

FLXL LEVEL 2 NODE 29 SHOW

EVAP 0.01 DEPTH 10. ELEM ALL
GOTI

! RECHARGE AT NODE 29 WITH ET SHOWN BELOW SHOULD BE 1.3424 CFD.

FLXL LEVEL 2 NODE 29 SHOW

! THE ALGEBRAIC SUM OF THE EVAPORATION AND THE RECHARGE
! IS 1.3424 AT NODE 29. THUS THE ET IS 1.5 - 1.3424 = 0.1576.
! TO CHECK THIS VALUE BY HAND INFORMATION IS NEEDED FROM THE
! SIMULATION REGARDING HEAD AND GROUND SURFACE ELEVATION AT NODE 29.

! THE HEAD AT NODE 29 (PH) IS (AT BEGINNING OF TIME STEP):

PH3 LEVEL 2 SHOW NODE 29

! THE GROUND SURFACE (GS) IS:

ELEV LEVEL 2 SHOW NODE 29

! THE EVAPORATION POTENTIAL (EP) AT NODE 29 IS 0.5 (AREA=50. * RATE=.01)
! THE ACTUAL EVAPORATION SHOULD THEREFORE BE
! $[1 - (GS - PH) / DEPTH] * EP = [1 - (80 - 73.152) / 10] * 0.5$
! = .1576 (QED)

END

Grid File DFTC12.GRF

33	40				
1	0	10.0	18	50.0	0
2	0	5.0	19	60.0	10.0
3	0	0	20	60.0	5.0
4	10.0	10.0	21	60.0	0
5	10.0	5.0	22	70.0	10.0
6	10.0	0	23	70.0	5.0
7	20.0	10.0	24	70.0	0
8	20.0	5.0	25	80.0	10.0
9	20.0	0	26	80.0	5.0
10	30.0	10.0	27	80.0	0
11	30.0	5.0	28	90.0	10.0
12	30.0	0	29	90.0	5.0
13	40.0	10.0	30	90.0	0
14	40.0	5.0	31	100.0	10.0
15	40.0	0	32	100.0	5.0
16	50.0	10.0	33	100.0	0
17	50.0	5.0			

Test Summary: DYNFLOW - Version 5.18 (September 1996; Built June 4, 1997)

1	5	4	1	21	20	19	16
2	2	5	1	22	17	20	16
3	3	5	2	23	18	20	17
4	3	6	5	24	18	21	20
5	8	7	4	25	23	22	19
6	5	8	4	26	20	23	19
7	6	8	5	27	21	23	20
8	6	9	8	28	21	24	23
9	11	10	7	29	26	25	22
10	8	11	7	30	23	26	22
11	9	11	8	31	24	26	23
12	9	12	11	32	24	27	26
13	14	13	10	33	29	28	25
14	11	14	10	34	26	29	25
15	12	14	11	35	27	29	26
16	12	15	14	36	27	30	29
17	17	16	13	37	32	31	28
18	14	17	13	38	29	32	28
19	15	17	14	39	30	32	29
20	15	18	17	40	30	33	32

Output File DFTC12.OUT

```
>>>> THIS PROGRAM IS THE CONFIDENTIAL AND PROPRIETARY INFORMATION OF <<<<<
>>>> CAMP DRESSER & MCKEE INC. COPYRIGHT (C) 1990 CAMP DRESSER & MCKEE INC. <<<<<
>>>> AS AN UNPUBLISHED WORK. ALL RIGHTS RESERVED. <<<<<
```

```
--- THIS RUN OF DYNFLOW5
--- VERSION 5.18E (ICCG SOLVER) SEPT. 1996
--- STARTED AT 1/ 7/98 18:53: 3.
--- ALL OUTPUT WILL BE TAGGED WITH THIS DATE/TIME.
```

TITL

```
ENTER THREE TITLE LINES ("!" FOR NO CHANGE) :
TITLE LINE #1 :
TITLE LINE #2 :
TITLE LINE #3 :
```

```
HERE IS HOW THE TITLE LINES LOOK --
VERIFICATION CASE NO. 12 -- EVAPORATION AND RECHARGE
STEADY-STATE UNCONFINED ONE-DIMENSIONAL FLOW WITH RECHARGE AND EVAPORATION
RUN BY PVDH, BOULDER CO.
```

TEXT

```
THIS TEST CASE DEMONSTRATES THAT APPROPRIATE
RECHARGE AND EVAPORATION TERMS ARE ADDED TO THE SOLUTION.
```

ENDT

```
! INITIAL MODEL SETUP
```

```
GRID READ DFTC12.GRF FORMatted
```

```
GRID INFO. NOW BEING PROCESSED ...
```

Test Summary: DYNFLOW - Version 5.18 (September 1996; Built June 4, 1997)

--- GRID DATA NOW BEING CLEARED,
--- PRIOR TO FILE READING.

--- GRID WAS PROCESSED OK.

*** WRITING GRID DATA TO FILE "GRID.SUM"

LEVEL 2.
FREE
ELEM 301.
ELEV 0. LEVELSING 1
ELEV 80. LEVELSING 2
INIT 75.
INIT 50. LEVELALL NODE RANGE 1 3
FIX LEVELALL NODE RANGE 1 3
FIX LEVELALL NODE RANGE 31 33
DT 0.
TOL 0001.
ITER 100.

*** SPECIFIED ITERATION LIMIT : 100
*** MUST EXCEED ZERO,
*** BUT NOT THE MAX. ALLOWED : 30
*** CURRENT VALUE RETAINED : 5

!PRAL

NOPR

*** NO DEFAULT PRINT FOR THIS RUN
*** USE "SUMM", "ERRO", AND "PRIN" COMMANDS
*** FOR RESULTS AS REQUIRED.

! THE FIRST CASE SETS THE ELEMENT RECHARGE
! USING THE ORIGINAL METHOD OF ASSIGNMENT
! OF RECHARGE THROUGH THE 'PROP' COMMAND.
! THE GRID AREA IS 1000 FT.SQ. AND THE
! RECHARGE IS 0.01 FT/D GIVING A TOTAL FLUX
! OF 10 CFD

PROP

ENTER PROPERTIES IN FOLLOWING ORDER --
#,KXX,KYY,KZZ,STORATIVITY,SPEC. YIELD,RECHARGE,THETA,PHI,SPEC. GRAV.,EFF. STRESS:
1,1.,1.,100.,0.,0.,0.01,0.,0.

GOTIL 0.

*** I.C.C.G. SOLVER SELECTED

CG HAS CONVERGED IN 16 ITERATIONS
TIME: .00, ITER.: 1, MAX.ERR.: 22.03, AT NODE: 6, LEVEL: 1, TOL.: 1.00

CG HAS CONVERGED IN 16 ITERATIONS
TIME: .00, ITER.: 2, MAX.ERR.: 1.02, AT NODE: 16, LEVEL: 2, TOL.: 1.00

CG HAS CONVERGED IN 16 ITERATIONS
TIME: .00, ITER.: 3, MAX.ERR.: .04, AT NODE: 7, LEVEL: 2, TOL.: 1.00

!PRIN

! THE COMPUTED TOTAL RECHARGE SHOWN BELOW SHOULD BE 10 CFD (FIRST LINE).

FLXL LEVEL 2 SUM

*** THE SUM OF THE SPECIFIED VALUES IS: 9.999999
*** THE AVERAGE OF THE SPEC. VALUES IS: 3.030303E-01
*** THE MAXIMUM OF THE SPEC. VALUES IS: 5.000000E-01
*** THE MINIMUM OF THE SPEC. VALUES IS: 8.333334E-02
*** THE SUM OF THE POSITIVE VALUES IS: 9.999999
*** THE SUM OF THE NEGATIVE VALUES IS: 0.000000E+00

Test Summary: DYNFLOW - Version 5.18 (September 1996; Built June 4, 1997)

! THE SECOND CASE USES THE 'RECH' COMMAND WITH
! THE ELEMENT SPECIFICATION. THIS RECHARGE WILL BE ADDED
! TO THE ORIGINAL VALUE GIVEN THROUGH THE 'PROP' COMMAND
! TO GIVE A TOTAL OF 20 CFD.

RECH 0.01 ELEM ALL
GOTI 0.
*** I.C.C.G. SOLVER SELECTED

CG HAS CONVERGED IN 16 ITERATIONS
TIME: .00, ITER.: 1, MAX.ERR.: .20, AT NODE: 16, LEVEL: 2, TOL.: 1.00

!PRIN

! THE COMPUTED TOTAL RECHARGE SHOWN BELOW SHOULD BE 20 CFD (FIRST LINE).

FLXL LEVEL 2 SUM

*** THE SUM OF THE SPECIFIED VALUES IS: 19.999990
*** THE AVERAGE OF THE SPEC. VALUES IS: 6.060604E-01
*** THE MAXIMUM OF THE SPEC. VALUES IS: 9.999995E-01
*** THE MINIMUM OF THE SPEC. VALUES IS: 1.666666E-01
*** THE SUM OF THE POSITIVE VALUES IS: 19.999990
*** THE SUM OF THE NEGATIVE VALUES IS: 0.000000E+00

! THE THIRD CASE USES THE 'RECH' COMMAND WITH
! THE NODE SPECIFICATION. THE NUMBER OF NODES IS
! 33 SO THAT THE TOTAL RECHARGE WILL NOW BE
! 43 CFD (10 + 33*1.0). NOTE THAT THE 'RECH' COMMAND
! REPLACES THE ORIGINAL VALUES SET BY THE 'RECH'
! COMMAND BEFORE.

RECH 1.0 NODE ALL
GOTI 0.
*** I.C.C.G. SOLVER SELECTED

CG HAS CONVERGED IN 16 ITERATIONS
TIME: .00, ITER.: 1, MAX.ERR.: .40, AT NODE: 16, LEVEL: 2, TOL.: 1.00

!PRIN

! THE COMPUTED TOTAL RECHARGE SHOWN BELOW SHOULD BE 43 CFD (FIRST LINE).

FLXL LEVEL 2 SUM

*** THE SUM OF THE SPECIFIED VALUES IS: 43.000000
*** THE AVERAGE OF THE SPEC. VALUES IS: 1.303030
*** THE MAXIMUM OF THE SPEC. VALUES IS: 1.500000
*** THE MINIMUM OF THE SPEC. VALUES IS: 1.083333
*** THE SUM OF THE POSITIVE VALUES IS: 43.000000
*** THE SUM OF THE NEGATIVE VALUES IS: 0.000000E+00

! THE FOURTH CASE USES THE 'EVAP' COMMAND FOR ALL NODES.
! TO CHECK ITS OPERATION, THE RECHARGE AT NODE 29 (LOCATED ALONG THE
! CENTER AXIS OF THE GRID) WITH NO EVAPORATION SHOULD BE 1.5 CFD.
! THE RECHARGE AT NODE 29 WITH EVAPORATION SHOULD BE 1.3424 CFD.

! RECHARGE AT NODE 29 WITHOUT EVAPORATION SHOWN BELOW SHOULD EQUAL 1.5 CFD

FLXL LEVEL 2 NODE 29 SHOW
2, 29, 1.500000 , .000000

EVAP 0.01 DEPTH 10. ELEM ALL
GOTI
*** I.C.C.G. SOLVER SELECTED

Test Summary: DYNFLOW - Version 5.18 (September 1996; Built June 4, 1997)

CG HAS CONVERGED IN 16 ITERATIONS
TIME: .00, ITER.: 1, MAX.ERR.: .02, AT NODE: 24, LEVEL: 2, TOL.: 1.00

! RECHARGE AT NODE 29 WITH ET SHOWN BELOW SHOULD BE 1.3424 CFD.

FLXL LEVEL 2 NODE 29 SHOW
2, 29, 1.342437, .0000000

! THE ALGEBRAIC SUM OF THE EVAPORATION AND THE RECHARGE
! IS 1.3424 AT NODE 29. THUS THE ET IS $1.5 - 1.3424 = 0.1576$.
! TO CHECK THIS VALUE BY HAND INFORMATION IS NEEDED FROM THE
! SIMULATION REGARDING HEAD AND GROUND SURFACE ELEVATION AT NODE 29.

! THE HEAD AT NODE 29 (PH) IS (AT BEGINNING OF TIME STEP):

PH3 LEVEL 2 SHOW NODE 29
2, 29, 73.15127, .0000000

! THE GROUND SURFACE (GS) IS:

ELEV LEVEL 2 SHOW NODE 29
2, 29, 80.00000, .0000000

! THE EVAPORATION POTENTIAL (EP) AT NODE 29 IS 0.5 (AREA=50. * RATE=.01)
! THE ACTUAL EVAPORATION SHOULD THEREFORE BE
! $[1 - (GS - PH) / DEPTH] * EP = [1 - (80 - 73.152) / 10] * 0.5$
! = .1576 (QED)

END

--- THIS RUN OF DYNFLOW5
--- VERSION 5.18E (ICCG SOLVER) SEPT. 1996
--- WHICH STARTED AT 1/ 7/98 18:53: 3
--- IS ENDING AT 1/ 7/98 18:53: 5

Test 14: ANISOTROPY

test #: DFTC14

command file: DFTC14.CFI

grid file: DFTC14.GRF

output file: DFTC14.OUT

date of test execution: April 1, 1998

title: Radial confined transient flow with anisotropy

description: Transient flow towards a fully penetrating pumping well with a constant discharge rate in an anisotropic, homogeneous, confined porous medium of infinite extent and constant thickness (see figure B14-1 and B14-2).

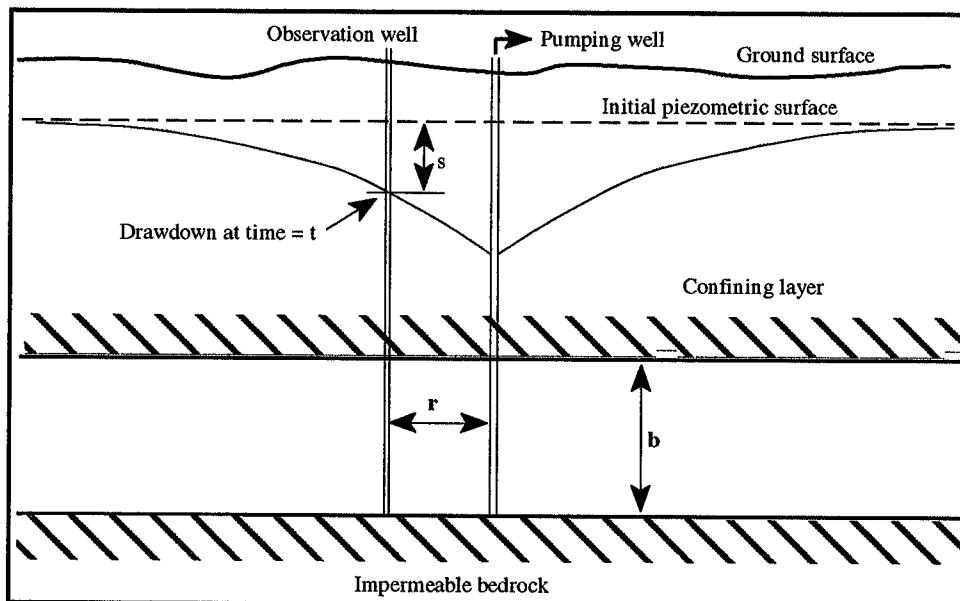


Figure B14-1.

tested functions: Transient horizontal flow under confined anisotropic conditions; effects of discharging well; influence of lateral fixed head boundary condition.

model domain: *Analytical solution*: infinite horizontal extent, aquifer thickness $b = 10$ ft; *numerical model*: 90° segment of radial domain with radius = 12,000 ft.

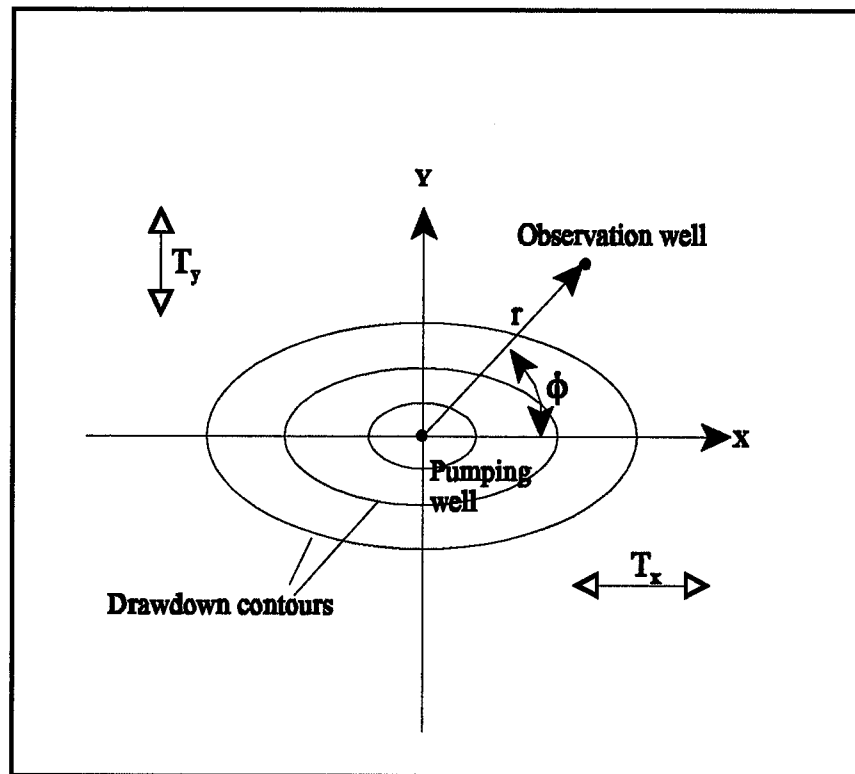


Figure B14-2

boundary conditions: Fixed head at outer radial boundary ($R=12,000$ ft) is $h_R = 100$ ft; well discharge rate $Q = 1000$ ft^3/d is distributed as a specified flux boundary condition in the central node; remaining boundary nodes are specified as no-flow boundaries.

benchmark: Hantush-Thomas analytical solution for drawdown with given transmissivities, storativity and pumping rate:

$$s = \frac{Q}{4\pi\sqrt{(T_x T_y)}} W(u)$$

where

$$u = \frac{r^2 S}{4tT_r}$$

and

$$T_r = \frac{T_x}{\cos^2\phi + \frac{T_x}{T_y}\sin^2\phi}$$

where, Q = flow rate [L^3/T], T_x , T_y = transmissivity in x-direction and y-direction, respectively [L^2/T], S = storativity [fraction], r = radial distance from pumping well to observation well [L], t = time since start of pumping [T], s = drawdown in observation well with respect to prepumping horizontal piezometric surface [L], ϕ = the angle between the x-axis and the line between the pumping well and the observation well, and W = the Theis Well Function [dimensionless]. Transmissivities are calculated from respective hydraulic conductivities multiplied by aquifer thickness. Calculation are made for observation wells at $r = 150, 250, 400, 550,$ and 900 ft from the well, respectively.

grid: single layer full quadrant (90°) grid with two levels (see Figure B14-3); in plan view, the grid consists of 169 nodes defining 282 elements.

initial conditions: $h=100$ ft

time-stepping: Model time steps of 0.1 days; comparison at $t=1$ day.

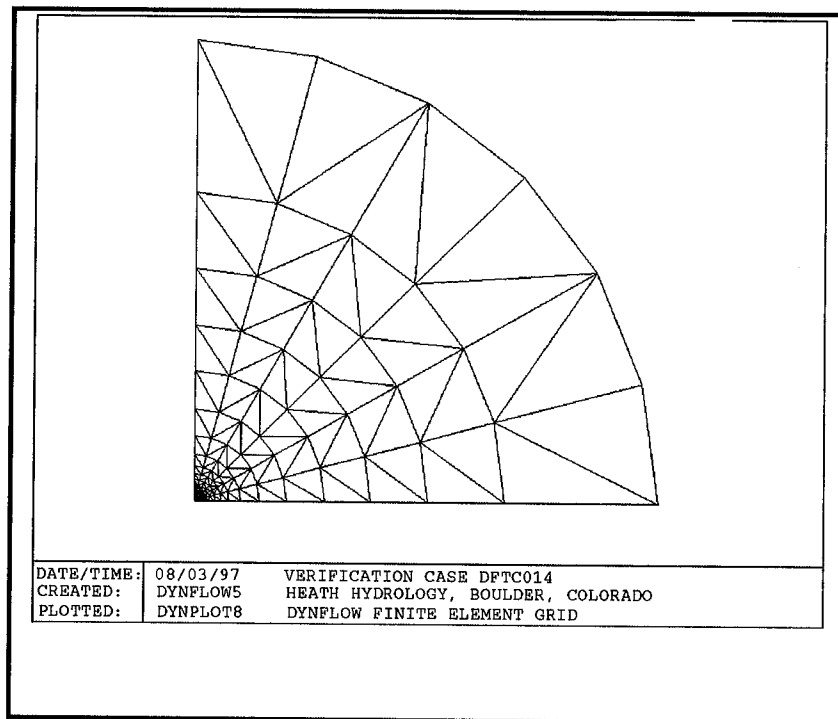


Figure B14-3.

system parameters: hydraulic conductivity $k_x = 1000$ m/d; $K_y = 100$ m/d; $k_z = 100$ ft/d; specific storativity = 0.00001 ft $^{-1}$ (storage coefficient = 0.001); aquifer specific yield = 0.10 (not used).

control parameters: Tolerance = .0001 ft; alpha = 1.5 (relaxation factor); acce = 1.5; max. # outer iterations = 30; max. # inner iterations = 90

solver: ICCG

test performed by: Problem set up for numerical code prepared by code developers; code run and benchmark comparison performed by test report author.

type of comparison: Tabular listing of heads (see Table B14-1); no statistical measures calculated.

Table B14-1.

angle ϕ degrees	Drawdown (ft)									
	r = 150 ft		r = 250 ft		r = 400 ft		r = 550 ft		r = 900 ft	
	BM	DF	BM	DF	BM	DF	BM	DF	BM	DF
0	1.16	1.10 (58)	0.91	0.86 (72)	0.68	0.65 (86)	0.52	0.50 (93)	0.31	0.29(107)
45	0.74	0.72 (61)	0.49	0.49 (75)	0.29	0.29 (89)	0.17	0.18 (96)	0.05	0.05(110)
90	0.59	0.59 (64)	0.36	0.36 (78)	0.18	0.19 (92)	0.09	0.09 (99)	0.01	0.01(113)
BM = Benchmark; DF = Dynflow (node number between brackets)										

# iterations:	27
influx [ft ³ /d]:	257.429
outflux [ft ³ /d]:	257.464
total mass balance error:	.01 %

performance notes: The computed heads compare very well with the benchmark, both in the direction of the X-axis and in the direction perpendicular to the x-axis.

Command File DFTC14.CFI

OUTPUT DFTC14.OUT

TITLE
 VERIFICATION CASE NO. 14 -- ANISOTROPY; RUN BY PVDH, BOULDER, CO.
 TRANSIENT CONFINED FLOW TOWARDS A FULLY PENETRATING WELL IN AN
 ANISOTROPIC AQUIFER TX>TY.

GRID READ DFTC14.GRF FORM

LEVEL 2.
 FREE
 ELEV 0. LEVELSING 1
 ELEV 10. LEVELSING 2
 PROP
 1,100.,10.,100.,0.0001,0.10,0.,0.,0.
 ELEM 301.

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

```

INIT 100.
FIX
FLUX -125. LEVELALL NODE RANGE 163 169
DT 0.1 LEVELALL NODE SING 1
TOL .0001
ACCE 1.5
PRAL
ALPHA 1.5
ITIN 90.
ITER 30.
NOPR
GOTIL 1
PRINT

XCFI
  
```

Grid File DFTC14.GRF

169	282			
1	.000	.000		
2	4.000	.000	47	63.64
3	3.864	1.035	48	45.00
4	3.464	2.000	49	23.29
5	2.828	2.828	50	.000
6	2.000	3.464	51	120.0
7	1.035	3.864	52	115.9
8	.000	4.000	53	103.9
9	10.00	.000	54	84.85
10	9.659	2.588	55	60.00
11	8.660	5.000	56	31.06
12	7.071	7.071	57	.000
13	5.000	8.660	58	150.0
14	2.588	9.659	59	144.9
15	.000	10.00	60	129.9
16	18.00	.000	61	106.1
17	17.39	4.659	62	75.00
18	15.59	9.000	63	38.82
19	12.73	12.73	64	.000
20	9.000	15.59	65	200.0
21	4.659	17.39	66	193.2
22	.000	18.00	67	173.2
23	30.00	.000	68	141.4
24	28.98	7.765	69	100.00
25	25.98	15.00	70	51.76
26	21.21	21.21	71	.000
27	15.00	25.98	72	250.0
28	7.765	28.98	73	241.5
29	.000	30.00	74	216.5
30	45.00	.000	75	176.8
31	43.47	11.65	76	125.0
32	38.97	22.50	77	64.70
33	31.82	31.82	78	.000
34	22.50	38.97	79	325.0
35	11.65	43.47	80	313.9
36	.000	45.00	81	281.5
37	65.00	.000	82	229.8
38	62.79	16.82	83	162.5
39	56.29	32.50	84	84.12
40	45.96	45.96	85	.000
41	32.50	56.29	86	400.0
42	16.82	62.79	87	386.4
43	.000	65.00	88	346.4
44	90.00	.000	89	282.8
45	86.93	23.29	90	200.0
46	77.94	45.00	91	103.5
			92	.000

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

93	550.0	.000	134	.000	2400.
94	531.3	142.4	135	3400.	.000
95	476.3	275.0	136	3284.	880.0
96	388.9	388.9	137	2944.	1700.
97	275.0	476.3	138	2404.	2404.
98	142.4	531.3	139	1700.	2944.
99	.000	550.0	140	880.0	3284.
100	700.0	.000	141	.000	3400.
101	676.1	181.2	142	4600.	.000
102	606.2	350.0	143	4443.	1191.
103	495.0	495.0	144	3984.	2300.
104	350.0	606.2	145	3253.	3253.
105	181.2	676.1	146	2300.	3984.
106	.000	700.0	147	1191.	4443.
107	900.0	.000	148	.000	4600.
108	869.3	232.9	149	6100.	.000
109	779.4	450.0	150	5892.	1579.
110	636.4	636.4	151	5283.	3050.
111	450.0	779.4	152	4313.	4313.
112	232.9	869.3	153	3050.	5283.
113	.000	900.0	154	1579.	5892.
114	1200.	.000	155	.000	6100.
115	1159.	310.6	156	8100.	.000
116	1039.	600.0	157	7824.	2096.
117	848.5	848.5	158	7015.	4050.
118	600.0	1039.	159	5728.	5728.
119	310.6	1159.	160	4050.	7015.
120	.000	1200.	161	2096.	7824.
121	1700.	.000	162	.001	8100.
122	1642.	440.0	163	12100.	.000
123	1472.	850.0	164	11688.	3132.
124	1202.	1202.	165	10479.	6050.
125	850.0	1472.	166	8556.	8556.
126	440.0	1642.	167	6050.	10479.
127	.000	1700.	168	3132.	11688.
128	2400.	.000	169	.001	12100.
129	2318.	621.2			
130	2078.	1200.			
131	1697.	1697.			
132	1200.	2078.			
133	621.2	2318.			

1	1	2	3	28	20	21	14
2	1	3	4	29	14	22	15
3	1	4	5	30	14	21	22
4	1	5	6	31	16	23	17
5	1	6	7	32	23	24	17
6	1	7	8	33	17	25	18
7	2	9	3	34	17	24	25
8	9	10	3	35	18	25	19
9	3	11	4	36	25	26	19
10	3	10	11	37	19	27	20
11	4	11	5	38	19	26	27
12	11	12	5	39	20	27	21
13	5	13	6	40	27	28	21
14	5	12	13	41	21	29	22
15	6	13	7	42	21	28	29
16	13	14	7	43	23	30	24
17	7	15	8	44	30	31	24
18	7	14	15	45	24	32	25
19	9	16	10	46	24	31	32
20	16	17	10	47	25	32	26
21	10	18	11	48	32	33	26
22	10	17	18	49	26	34	27
23	11	18	12	50	26	33	34
24	18	19	12	51	27	34	28
25	12	20	13	52	34	35	28
26	12	19	20	53	28	36	29
27	13	20	14	54	28	35	36

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

55	30	37	31	125	70	78	71
56	37	38	31	126	70	77	78
57	31	39	32	127	72	79	73
58	31	38	39	128	79	80	73
59	32	39	33	129	73	81	74
60	39	40	33	130	73	80	81
61	33	41	34	131	74	81	75
62	33	40	41	132	81	82	75
63	34	41	35	133	75	83	76
64	41	42	35	134	75	82	83
65	35	43	36	135	76	83	77
66	35	42	43	136	83	84	77
67	37	44	38	137	77	85	78
68	44	45	38	138	77	84	85
69	38	46	39	139	79	86	80
70	38	45	46	140	86	87	80
71	39	46	40	141	80	88	81
72	46	47	40	142	80	87	88
73	40	48	41	143	81	88	82
74	40	47	48	144	88	89	82
75	41	48	42	145	82	90	83
76	48	49	42	146	82	89	90
77	42	50	43	147	83	90	84
78	42	49	50	148	90	91	84
79	44	51	45	149	84	92	85
80	51	52	45	150	84	91	92
81	45	53	46	151	86	93	87
82	45	52	53	152	93	94	87
83	46	53	47	153	87	95	88
84	53	54	47	154	87	94	95
85	47	55	48	155	88	95	89
86	47	54	55	156	95	96	89
87	48	55	49	157	89	97	90
88	55	56	49	158	89	96	97
89	49	57	50	159	90	97	91
90	49	56	57	160	97	98	91
91	51	58	52	161	91	99	92
92	58	59	52	162	91	98	99
93	52	60	53	163	93	100	94
94	52	59	60	164	100	101	94
95	53	60	54	165	94	102	95
96	60	61	54	166	94	101	102
97	54	62	55	167	95	102	96
98	54	61	62	168	102	103	96
99	55	62	56	169	96	104	97
100	62	63	56	170	96	103	104
101	56	64	57	171	97	104	98
102	56	63	64	172	104	105	98
103	58	65	59	173	98	106	99
104	65	66	59	174	98	105	106
105	59	67	60	175	100	107	101
106	59	66	67	176	107	108	101
107	60	67	61	177	101	109	102
108	67	68	61	178	101	108	109
109	61	69	62	179	102	109	103
110	61	68	69	180	109	110	103
111	62	69	63	181	103	111	104
112	69	70	63	182	103	110	111
113	63	71	64	183	104	111	105
114	63	70	71	184	111	112	105
115	65	72	66	185	105	113	106
116	72	73	66	186	105	112	113
117	66	74	67	187	107	114	108
118	66	73	74	188	114	115	108
119	67	74	68	189	108	116	109
120	74	75	68	190	108	115	116
121	68	76	69	191	109	116	110
122	68	75	76	192	116	117	110
123	69	76	70	193	110	118	111
124	76	77	70	194	110	117	118

Test Summary: DYNFLOW - Version 5.18 (September 1996, Built June 4, 1997)

195	111	118	112	243	139	146	140
196	118	119	112	244	146	147	140
197	112	120	113	245	140	148	141
198	112	119	120	246	140	147	148
199	114	121	115	247	142	149	143
200	121	122	115	248	149	150	143
201	115	123	116	249	143	151	144
202	115	122	123	250	143	150	151
203	116	123	117	251	144	151	145
204	123	124	117	252	151	152	145
205	117	125	118	253	145	153	146
206	117	124	125	254	145	152	153
207	118	125	119	255	146	153	147
208	125	126	119	256	153	154	147
209	119	127	120	257	147	155	148
210	119	126	127	258	147	154	155
211	121	128	122	259	149	156	150
212	128	129	122	260	156	157	150
213	122	130	123	261	150	158	151
214	122	129	130	262	150	157	158
215	123	130	124	263	151	158	152
216	130	131	124	264	158	159	152
217	124	132	125	265	152	160	153
218	124	131	132	266	152	159	160
219	125	132	126	267	153	160	154
220	132	133	126	268	160	161	154
221	126	134	127	269	154	162	155
222	126	133	134	270	154	161	162
223	128	135	129	271	156	163	157
224	135	136	129	272	163	164	157
225	129	137	130	273	157	165	158
226	129	136	137	274	157	164	165
227	130	137	131	275	158	165	159
228	137	138	131	276	165	166	159
229	131	139	132	277	159	167	160
230	131	138	139	278	159	166	167
231	132	139	133	279	160	167	161
232	139	140	133	280	167	168	161
233	133	141	134	281	161	169	162
234	133	140	141	282	161	168	169
235	135	142	136				
236	142	143	136				
237	136	144	137				
238	136	143	144				
239	137	144	138				
240	144	145	138				
241	138	146	139				
242	138	145	146				